

City of Keene Planning Board

AGENDA

Monday, September 8, 2025 5:30 PM City Hall, 2nd Floor Council Chambers

A. AGENDA ITEMS

- 1) Call to Order Roll Call
- 2) Boundary Line Adjustment
 - a) PB-2025-16 Boundary Line Adjustment 124-126 & 130 Eastern Ave Applicant HG Johnson Real Estate, on behalf of owner Bishop 2024 Revocable Family Trust, proposes to transfer ~0.22-ac of land from the ~0.57-ac parcel at 130 Eastern Ave to the ~0.95-ac parcel at 124-126 Eastern Ave (TMP#s 588-031-000 & 588-032-000). Both parcels are located in the Low Density District.

3) Public Hearings

a) PB-2025-15 - Cottage Court Conditional Use Permit - Barn Conversion, 429 Elm St
 - Applicant HG Johnson Real Estate, on behalf of owner JC&C Rentals LLC, proposes to convert a barn into 2 dwelling units on the property at 429 Elm St (TMP# 520-005-000). The parcel is ~0.48-ac and is located in the Low Density District.

4) Continued Public Hearing

- a) PB-2024-20 Earth Excavation Permit Major Amendment & Hillside Protection Conditional Use Permit Gravel Pit, 21 & 57 Route 9 Applicant Granite Engineering LLC, on behalf of owner G2 Holdings LLC, proposes to expand the existing gravel pit located at 21 & 57 Route 9 (TMP#s 215-007-000 & 215-008-000). A Hillside Protection CUP is requested for impacts to steep slopes. Waivers are requested from Sections 25.3.1.D, 25.3.6, and 25.3.13 of the LDC related to the 250' surface water resource setback, toxic or acid forming materials, and the 5-ac excavation area maximum. The parcels are a combined ~109.1-ac in size and are located in the Rural District.
- 5) Staff Updates
- 6) New Business
- 7) <u>Upcoming Dates of Interest</u>
 - Joint Committee of the Planning Board and PLD September 8th, 7:30 PM
 - Planning Board Steering Committee September 16th, 12:00 PM

- Planning Board Site Visit September 24th, 8:00 AM <u>To Be Confirmed</u>
 Planning Board Meeting September 29th, 6:30 PM

B. MORE TIME ITEMS

- 1. Potential Modifications to the Site Plan Review Thresholds
- 2. Training on Site Development Standards Snow Storage & Landscaping

C. ADJOURNMENT

PB-2025-16 - BOUNDARY LINE ADJUSTMENT - 124-126 & 130 EASTERN AVE

Requests:

Applicant HG Johnson Real Estate, on behalf of owner Bishop 2024 Revocable Family Trust, proposes to transfer \sim 0.22-ac of land from the \sim 0.57-ac parcel at 130 Eastern Ave to the \sim 0.95-ac parcel at 124-126 Eastern Ave (TMP#s 588-031-000 & 588-032-000). Both parcels are located in the Low Density District.

Background:

The subject parcels located on the east side of Eastern Ave., in southeastern Keene. ~1.700 ft from the Marlboro St. intersection to the south and ~350 ft from the Bellevue St intersection to the north. The property at 124-126 Eastern Ave. contains an existing three-unit family building, an existing single-family building, port, shed, and associated site improvements. There is a stream that runs along the northwest corner of the parcel and under Eastern Ave. The property at 130 Eastern Ave. contains an existing singledetached family building, garage, shed, and associated site improvements.



Fig 1: The subject properties at 124-126 & 130 Eastern Avenue outlined in yellow.

The purpose of the application is to adjust the common property boundary between the two subject parcels to accommodate the transfer of \sim 0.22 ac of land from 130 Eastern Ave. to 124-126 Eastern Ave. There is no new development proposed with this application.

Determination of Regional Impact:

After reviewing the application, staff have made a preliminary evaluation that the proposed boundary line adjustment does not appear to have the potential for "regional impact" as defined in RSA 36:55. The Board will need to make a final determination as to whether the proposal, if approved, could have the potential for regional impact.

Completeness:

The applicant has requested an exemption from submitting separate existing and proposed condition plans and all technical reports. After reviewing each request, Planning Staff have made the preliminary determination that granting the requested exemptions would have no bearing on the merits of the application and recommend that the Board accept the application as "complete."

Application Analysis:

<u>SECTION 20.2.1 – LOTS:</u> The applicant proposes to transfer \sim 0.22 ac of land from the \sim 0.57 ac parcel at 130 Eastern Ave to the \sim 0.99 ac parcel at 124-126 Eastern Ave. Following this transfer of land, both parcels will still comply with the zoning dimensional requirements for the Low Density District, as shown in Table 1. This standard appears to be met.

Table 1. Area of Land Affected by Proposed Boundary Line Adjustment			
Lot Sizes	124-126 Eastern Ave.	130 Eastern Ave.	
	Low Density	Low Density	
Required in District	10k SF (~0.23 ac)	10k SF (~0.23 ac)	
Before BLA	0.99 ac	0.57 ac	
Amount of Land Transferred	+0.22 ac	-0.22 ac	
After BLA Parcel Size	1.21 ac	0.34 ac	

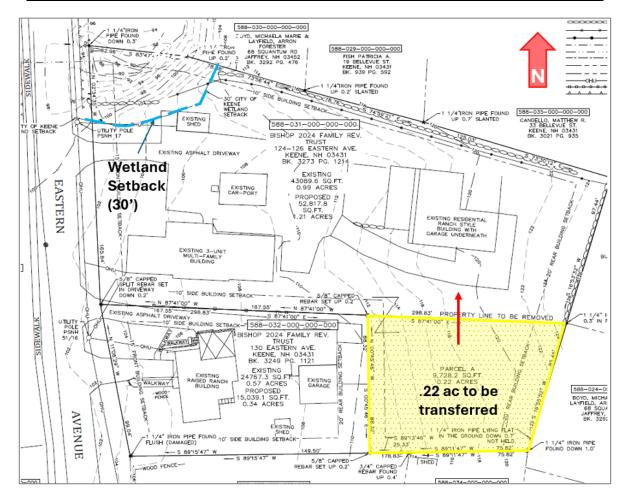


Fig 2: Proposed land to be transferred

<u>SECTION 20.2.2 – CHARACTER OF LAND FOR SUBDIVISION:</u> The plan shows the developed parcels gently sloping towards Eastern Ave with a low point located to the northern edge of 124-126 Eastern Ave. where the stream is located. A portion of the stream is located on the northwestern corner of the parcel, and the plan shows the delineated wetland and 30 ft surface water buffer. The portion of the land to be transferred is a gently sloping open lawn area. No development is currently proposed for this area. This standard appears to be met.

<u>SECTION 20.2.3 – SCATTERED OR PREMATURE DEVELOPMENT:</u> Both properties are developed with residential uses and associated site improvements. No new development is proposed with this application. It appears that this standard has been met.

<u>SECTION 20.2.4 – PRESERVATION OF EXISTING FEATURES:</u> There will be no impacts to existing site features as no additional development is proposed with the application. It appears that this standard has been met.

<u>SECTION 20.2.5 – MONUMENTATION:</u> The submitted plan shows that boundaries will be marked using 5/8" capped rebar posts that will be set at all corners. Planning Staff recommend that the Board include a condition of approval related to the inspection of the lot monuments or the submittal of a security to cover the cost of a lot monument inspection prior to the final approval of this application. This standard appears to be met.

<u>SECTION 20.2.6 – SPECIAL FLOOD HAZARD AREAS:</u> The plan states that the subject parcels are not located within a Special Flood Hazard Area as shown on the Federal Emergency Management Agency's (FEMA's) Flood Insurance Rate Maps (FIRM). This standard is not applicable.

<u>SECTION 20.2.7 – FIRE PROTECTION & WATER SUPPLY:</u> There is no development proposed that would require the installation of fire protection or additional water supply services. This standard is not applicable.

SECTION 20.2.8 – UTILITIES: Both subject parcels are served by municipal water and sewer service with no change proposed to those services. This standard is not applicable.

Recommended Motion:

If the Board is inclined to approve this request, the following motion is recommended:

"Approve PB-2025-16 as shown on the plan identified as "Boundary Line Adjustment Plan" prepared by Envirespect Land Services, LLC at a scale of 1 inch = 20 feet dated June 28, 2025 and last revised August 2025 with the following conditions precedent prior to final approval and signature of the plans by the Planning Board Chair:

- 1. Owners' signatures appear on the proposed BLA plan.
- 2. Submittal of two (2) mylar copies of the plans.
- 3. Submittal of a check in the amount of \$51 made out to the City of Keene to cover recording fees.
- 4. Inspection of the lot monuments by the Public Works Director, or their designee, following their installation, or the submittal of a security in a form and amount acceptable to the Public Works Director to ensure that the monuments will be set."



If you have questions about how to complete this form, please call: (603) 352-5440 or email: communitydevelopment@keenenh.gov

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SECTION 1: PROJEC	CT INFORMATION
PROJECT NAME: Boundary Line Adjustment - 124-126 Eastern Ave & 1	30 Eastern Ave
PROJECT ADDRESS(ES): 124-126 Eastern Ave., 130 Ea	astern Ave
SECTION 2: CONTA	CT INFORMATION
PROPERTY OWNER #1	PROPERTY OWNER #2
NAME/COMPANY: Bishop 2024 Revocable Trust	Bishop 2024 Revocable Trust
MAILING ADDRESS: 124 Eastern Ave., Keene, NH 03431	MAILING ADDRESS: 124 Eastern Ave., Keene, NH 03431
PHONE: 603-748-1232	PHONE: 603-748-1232
SIGNATURE: PRINTED NAME: Terry Bishop, Trustee	SIGNATURE: DUBLING PRINTED NAME: Terry Bishop, Trustee
APPLICANT / AUTHORIZED AGENT	FOR OFFICE USE ONLY:
NAME/COMPANY: George Hansel/HG Johnson Real Estate MAILING ADDRESS: 17 Flm St. Suite B202, Keepe, NH 03431	TAX MAP PARCEL #(s): 588-031-000- (124-126 Eastern Avc)
PHONE: 603-903-3677	588.032.000.(130 Eastern Ave)
george@hgjohnson.com SIGNATURE: PRINTED NAME: George Hansel	PARCEL SIZE: 0. 95 AC (124-126 EASTERN) 0.57 AC (130 EASTERN) ZONING: LOW DENSITY (Both) PROJECT #: PB-2025-10





7/15/2025

City of Keene, Community Development Department 3 Washington St., Keene, NH 03431 Attn: Planning Staff, Planning Board Chair

Re: 124-126 Eastern Ave., 130 Eastern Ave. – Narrative for Boundary Line Adjustment Review

Project Name: Boundary Line Adjustment - 124-126 Eastern Ave., 130 Eastern Ave.

Chair Farrington and Planning Staff,

HG Johnson Real Estate, on behalf of the property owner Bishop 2024 Revocable Trust, submit the following information to aid in the granting of a boundary line adjustment between 124-126 Eastern Avenue (TMP#: 588-031-000-000-000) and 130 Eastern Avenue (TMP#: 588-032-000-000-000). Both parcels are owned by the Bishop 2024 Revocable Trust and have separate access/frontage along Eastern Avenue. Both properties are served by city water and sewer. 124-126 Eastern Avenue currently has (2) buildings; (1) three-unit multifamily structure, and (1) single-family home. 130 Eastern Avenue has a single-family home and a detached garage.

No new development is anticipated with this request. This boundary line adjustment will transfer 0.22 acres (9,728.2 square feet) from 130 Eastern Avenue to 126-130 Eastern Avenue. The adjusted lots will both conform to the Low-Density zoning district requirements outlined in the Land Development Code (LDC).

The sizes of the existing and proposed lots are as follows:

124-126 Eastern Avenue

Existing: 0.99 acres (43,089.6 square feet), Proposed: 1.21 acres (52,817.8 square feet)

130 Eastern Avenue

Existing: 0.57 acres (24,767.3 square feet), Proposed: 0.34 acres (15,039.1 square feet)

This boundary line adjustment as proposed complies with Article 20 of the LDC. As no new development or changes of use are being considered, the Site Development Standards outlined in Article 21 of the LDC do not apply to this request.



We request exemptions from providing an existing conditions plan or a proposed conditions plan. The lot line proposed for adjustment is highlighted on the attached survey prepared by Envirospect Land Services LLC and Dated June 28, 2023.

Thank you for your consideration of this application. Please feel free to reach out to me with any questions or concerns.

Bestregards,

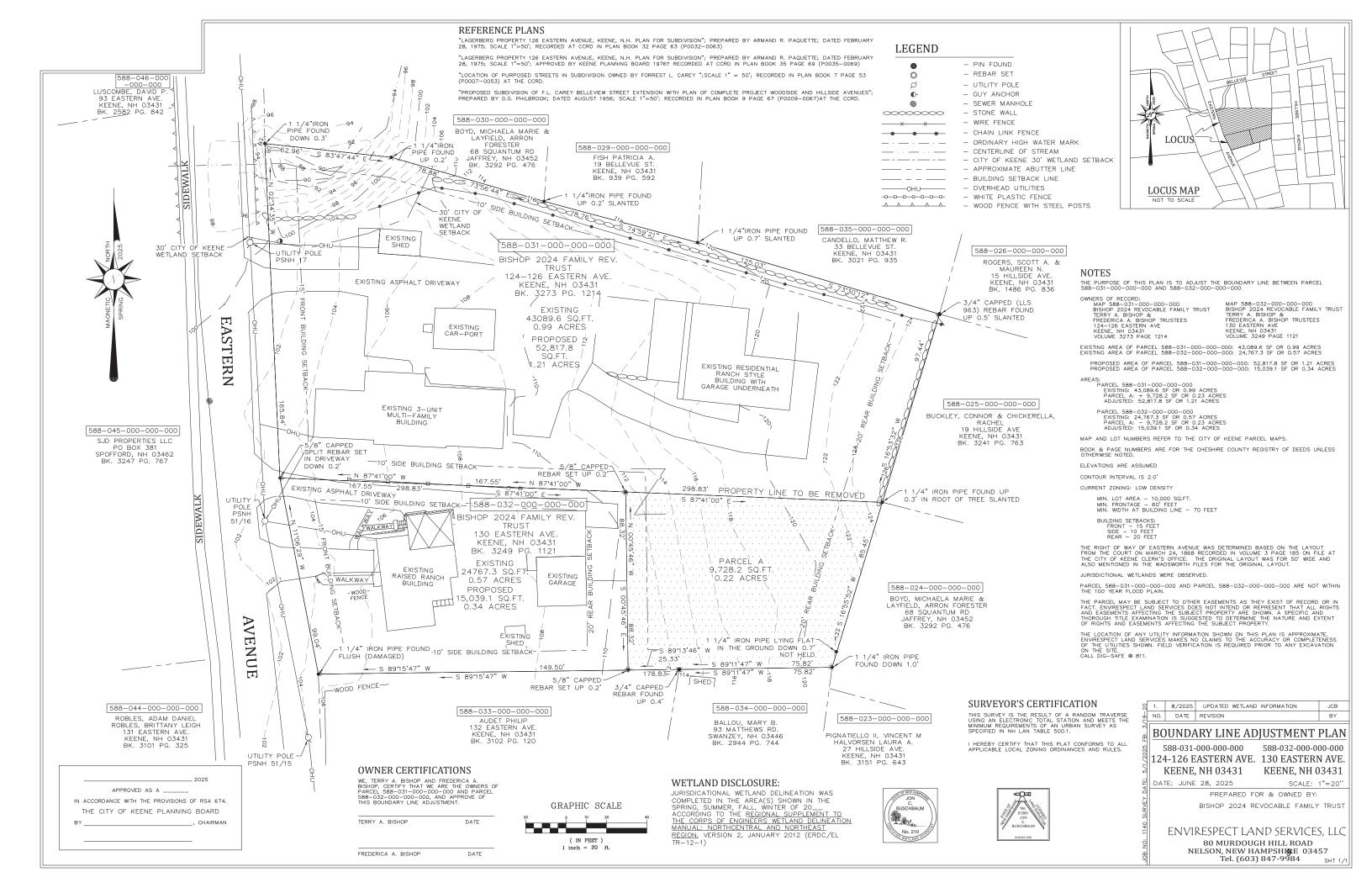
George Hansel, HG Johnson Real Estate

Submitted on behalf of:

Terry Bishop, Trustee, Bishop 2024 Revocable Trust

Attachments:

- 1) Boundary Line Adjustment Application
- 2) Abutter's list and mailing labels
- 3) Site Survey



PB-2025-20 - COTTAGE COURT CONDITIONAL USE PERMIT - 429 ELM ST

Request:

Applicant HG Johnson Real Estate, on behalf of owner JC&C Rentals LLC, proposes to convert a barn into 2 dwelling units on the property at 429 Elm St (TMP# 520-005-000). The parcel is \sim 0.48-ac and is located in the Low Density District.

Background:

The subject parcel is an existing 0.48 ac lot located on the eastern side of Elm St. directly across the street from Fuller Elementary School and ~330 ft from the Timberlane Dr. intersection. The property contains three existing detached single-family residences, a shed, barn, and associated site improvements includina U-shaped gravel а driveway with two street access connections.

The purpose of this application is to convert the existing ~1120 SF barn into a duplex with a two bed, one bath unit and a one bed one bath unit. This will increase the total count of residential units on the



Figure 1. Aerial image of 429 Elm St. (outlined in yellow).

property to five units. The property contains eight existing parking spaces where only five spaces are required. There is no proposed alteration to site besides the barn conversion. This project does not meet the threshold for site plan review.

Determination of Regional Impact:

After reviewing the application, staff have made a preliminary evaluation that the proposed duplex does not appear to have the potential for "regional impact" as defined in RSA 36:55. The Board will need to make a final determination as to whether the proposal, if approved, could have the potential for regional impact.

Completeness:

The applicant requests exemptions from submitting separate existing and proposed conditions plan, a grading plan, a lighting plan, a landscaping plan, and all technical reports. Staff recommend that the Board grant these exemptions and accept the application as "complete."

Application Analysis: The following is a review of the Cottage Court Overlay requirements.

17.5.1 Development Types Allowed

The proposal is to convert an existing barn into a duplex. The development types allowed under the Cottage Court Overlay for the Low-Density Zoning District include single-family, duplex, triplex, townhome, and dwelling above ground floor. The three existing detached single-family units were constructed between 1936-1940. The existing development pattern of the site is considered legal non-conforming as multiple principal uses are not permitted on a lot in a residential zoning district. Approval of the Cottage Court Conditional Use Permit will bring the property into conformance with the City's Zoning Regulations in terms of allowed uses. All requirements appear to be met.

17.5.3 Conditional Use Permit Standards

- A. <u>Dwelling unit size</u>: This standard requires a maximum average size of 1,250 SF gross floor area (gfa) and a maximum building footprint of 900 SF per unit. The proposed project will result in an average gfa of 900 SF per unit with 5 dwelling units. The total building footprint of all existing buildings is 3,185 SF with an average of 637 SF per unit. Each building footprint appears to be less than 900 SF. This standard has been met.
- B. <u>Parking:</u> This standard requires a minimum of one parking space per unit and a maximum of one parking space per bedroom. Each unit within the property has access to one of the eight existing parking spaces on site. This standard has been met.
- C. <u>Building Separation</u>: All buildings are existing on site. The conversion of the barn to a duplex will require a building permit application that meets all applicable building, fire, and life safety codes. This standard has been met.
- D. <u>Driveways</u>: The property contains an existing driveway that appears to be at least 20 ft wide to accommodate vehicular traffic to parking area. This standard appears to have been met.
- E. Internal Roads: No internal roads are proposed; this standard does not apply.
- F. <u>Screening</u>: This standard states that either a six-foot tall fence or a landscaped buffer is required for screening if the proposed building type (not density) is more intense than the adjacent building type. The proposed conversion of an existing barn to a duplex with no significant changes to the overall site does not constitute a more intense building type to the surrounding properties. This standard does not apply.

17.5.4 Architectural Guidelines

This section includes guidance to promote developments that are respectful of the context of the surrounding neighborhood. Photos of the existing barn and the other buildings on the property are included below.



Figure 2. Picture of existing barn



Figure 3. Picture of site from Elm St.



Figure 4. Picture of single-family unit 2



Figure 5. Picture of single-family unit 3

The applicant proposes the conversion of the existing barn into a duplex. The conversion will include the removal of the garage doors, adding new entrances to the western side of the building, enlargement of the existing dormers, and the installation of new insulated windows. The overall architectural style of the building is not proposed to be altered. The Board will need to determine if this standard has been met.

Recommended Motion:

If the Board is inclined to approve this request, the following motion is recommended:

Approve PB-2024-15 as shown on the site plan identified as "429 Elm Street, Keene" prepared by H.G. Johnson Real Estate at a scale of 1 inch = 15'8" dated July 8, 2025 and in the application materials received July 9 2025, and August 4, 2025, with the following conditions:

- 1. Prior to final approval and signature of the plans by the Planning Board Chair, the following conditions shall be met:
 - a. Owner's signature appears on the site plan.
 - b. Submittal of five (5) paper copies and one digital copy of the site plan.

Cottage Court Conditional Use Permit (CUP) Application

If you have questions about now to complete this joint	n, please call:	(603) 352-5440 or email: comm	unitydevelopment@keenenh.gov
SECTION	1: PROJEC	T INFORMATION	
PROJECT NAME: 429 Elm Street	NUMBER OF NEW DWELLING UNITS PROPOSED: 2		OSED: 2
	(Please note: Proposals that include the creation of 5 or more new units will require current Major Site Plan review. See the Major/Minor Site Plan application for addition information.)		of 5 or more new units will require con- inor Site Plan application for additional
PROJECT ADDRESS(ES): 429 Elm Street Keene, NH 03431	DOES THIS PROJECT INCLUDE A PROPOSAL TO SUBDIVIDE ONE OR MORE PAR- CELS? YES NO		
AVERAGE GROSS FLOOR AREA OF ALL PROPOSED UNITS (In SF):	(If yes, a Sub the Cottage additional in	Court application. See the Article 20	oubmitted and reviewed currently with of the Land Development Code (LDC) for
SECTION	2: CONTA	CT INFORMATION	
PROPERTY OWNER			PLICANT
NAME/COMPANY: Cheryl & Joe Bagster/JC&C Re	entals LLC	NAME/COMPANY: JC&	C Rentals LLC
MAILING ADDRESS: 83 Eastern Ave. Keene, N	H 03431	MAILING ADDRESS: 83 Easte	ern Ave. Keene, NH 03431
PHONE: 603-313-5909		PHONE: 603-31	3-5909
iccb00@aol.com	1	jccb00@	
SIGNATURE: Joe and Cheryl Bagster		SIGNATURE: Joe and Cheryl Bu	ngster
SIGNATURE: Joe and Cheryl Bagster PRINTED NAME: Joe and Cheryl Bagster		PRINTED NAME:	Cheryl Bagster
Joe and Cheryl Bagster		PRINTED NAME: Joe and	
Joe and Cheryl Bagster PRINTED NAME: Joe and Cheryl Bagster AUTHORIZED AGENT (if different than Owner/Applicant) NAME/COMPANY: George Hansel/HG Johnson R		Joe and FOR OFF	Cheryl Bagster
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Joe and Cheryl Bagster PRINTED NAME: Joe and Cheryl Bagster AUTHORIZED AGENT (if different than Owner/Applicant) NAME/COMPANY: George Hansel/HG Johnson Remailing Address: 17 Elm Street, Suite 202B, Keene,	eal Estate	PRINTED NAME: Joe and FOR OFF TAX MAP PARCEL #(s): 5 2 0 - 0 0 5 - 0 0 0 - PARCEL SIZE: O. 48 ZONING DISTRICT:	Cheryl Bagster ICE USE ONLY:
Joe and Cheryl Bagster PRINTED NAME: Joe and Cheryl Bagster AUTHORIZED AGENT (if different than Owner/Applicant) NAME/COMPANY: George Hansel/HG Johnson Remailing Address: 17 Elm Street, Suite 202B, Keene, PHONE: 603–903–3677	eal Estate	PRINTED NAME: Joe and FOR OFF TAX MAP PARCEL #(s): 5 2 0 - 0 0 5 - 0 0 0 - PARCEL SIZE: O. 48	Cheryl Bagster ICE USE ONLY: DATE STAMP:



DECEIVE JUL **0 9** 2025 By_____

7/8/2025

City of Keene, Community Development Department 3 Washington St., Keene, NH 03431 Attn: Evan Clements, Community Development Department

Re: 429 Elm Street – Narrative for Cottage Court CUP Review

Project Name: 429 Elm Street Project

Mr. Clements,

HG Johnson Real Estate, on behalf of the property owner JC & C Rentals, submit the following information to aid in the granting of a conditional use permit (CUP) to utilize the Cottage Court Overlay district for proposed work on the lot located at 429 Elm Street (TMP#: 520-005-000-000-000).

In accordance with the Conditional Use Permit Standards 17.5.5.B described in the Land Development Code, we offer the following analysis to aid in the consideration of our request:

Written Narrative

The property currently has (3) buildings that contain (3) housing units. An unfinished barn and shed are also on the property. This project will involve renovating the interior of the existing barn to create (2) new residential apartments: (1) two-bed, one-bath units and (1) one-bed, one-bath unit. To accommodate this change, new entrances will be added to the western side of the building. Additionally, the existing garage doors will be eliminated, existing dormers will be enlarged or replaced, and new insulated windows will be installed. No other exterior modifications are planned as part of this project. The entire property is served by city water and sewer.

There are (8) existing parking spaces on the property. Once complete, there will be (5) housing units on site with an average gross living area per unit of 900 SF. The total footprint of all buildings on the site is 3,185 SF (637 SF per unit). The parcel is 0.48 acres in size and located in the Low Density (LD) zoning district.

In accordance with the Site Development Standards described in Article 21 of the Land Development Code, we offer the following analysis to aid in the consideration of our request:

21.2 Drainage & Stormwater Management

The existing drainage patterns on the site will be maintained and no major exterior improvements are planned beyond the addition of a new entrance on the southwest side of the building.

21.3 Sediment & Erosion Control See above.



21.4 Snow Storage & Removal

Snow will continue to be stored in various locations on the property.

21.5 Landscaping

No changes in landscaping are anticipated.

21.6 Screening

No additional screening is anticipated.

21.7 Lighting

New porch lights will be installed at the new entrances. The LED fixture will be dark skies compliant, with a 3000K warm white color temperature (cut sheet attached).

21.8 Sewer & Water

The property is currently served by (1) 6" sewer line and (1) ¾" potable water line connecting to Elm Street. A new water/sewer line will be extended from Elm Street to the barn under the existing driveway. A design flow calculation for the parcel post-development is in Table 2-1 below.

Table 2-1

429 Elm Street

Sewer/Water Design Flow Calculations

(000)			
Units	Bedrooms	Bathrooms	Design Flow (GPD)
1	1	1	150
1	2	1	225
2	3	2	375
1	2	1	225
1	2	1	225
1	2	1	225
3	6	3	675
		Total Combined	1050
		Net Change	375
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^{*}Unit design flows based on 150 GPD for new one-bedroom units

As a result of this renovation, we anticipate a net increase of water usage and sewer discharge on the site to be no more than 375 GPD (with peak usage occurring in the early morning and evening hours). Overall, we anticipate that the impact on city water and sewer utilities from this project will be negligible.

^{*}Unit design flows for 2-3 bedroom units based on NH DES Rules, ENV-Wq 1008, Page 55:



21.9 Traffic & Access Management

After consulting the ITE Trip Generation Manual, traffic from the new development is estimated to be between 1-2 vehicle trips per hour. We expect the net difference in trips to and from the site before and after this development to be negligible. Additionally, the trip estimates for these additional units are far below the 100 vehicle trips per hour threshold that would constitute a noticeable impact, based on current ITE and NHDOT methodologies. In other words, standard traffic engineering practice suggests that the proposed development would be expected to result in negligible impacts to the adjacent roadway system.

21.10 Filling & Excavation

N/A. No excavation is anticipated as part of this development.

21.11 Surface Waters & Wetlands

There are wetlands approximately 180' from the rear property line. This proposal will not impact any surface waters or wetlands.

21.12 Hazardous & Toxic Materials

N/A

21.13 Noise

Following construction, no additional noise that is inconsistent with residential uses that already exist on the site and in the surrounding area.

21.14 Architecture & Visual Appearance

No substantial changes to the architecture and visual appearance of the building are anticipated, beyond the addition of new entryways and porch light(s) near the back of the building, replacement of the dormers and windows, and elimination of the garage doors. The new porch light(s) will be a dark skies compliant LED fixture that emits 3000K of warm white light. The new entryway will closely match the existing building features.

We look forward to being granted permission to proceed with this exciting project that will add (4) new affordable housing units to the city's housing stock. Please reach out to me with any questions or concerns. Thank you.

Best regards,

George Hansel

George Hansel, HG Johnson Real Estate Attachments:

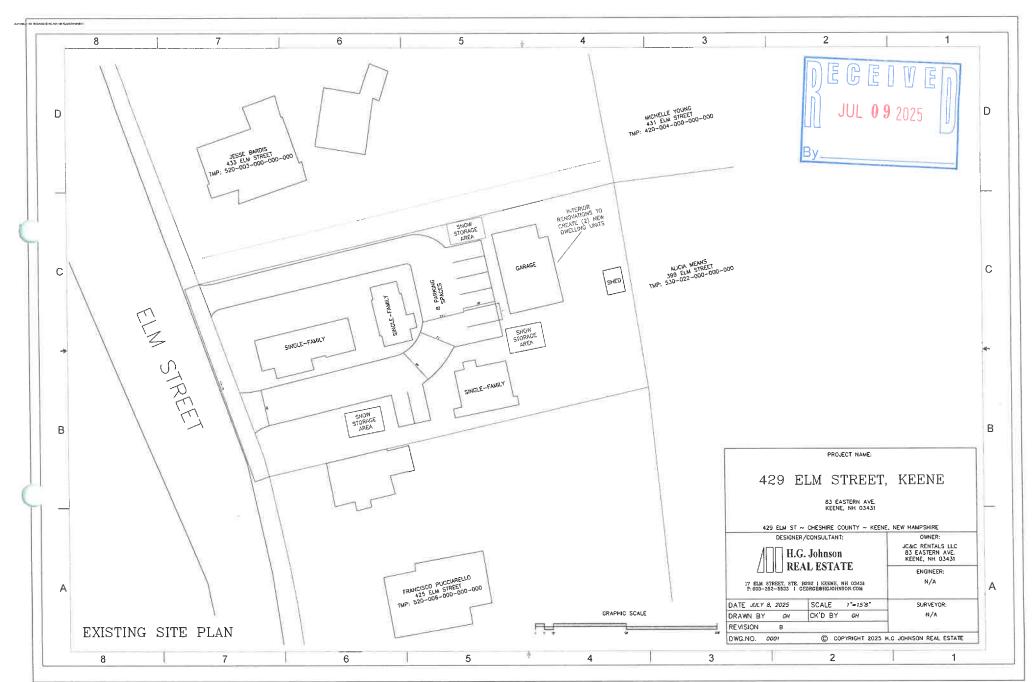
- 1) Cottage Court CUP Application
- 2) Plan Set 429 Elm Street Project

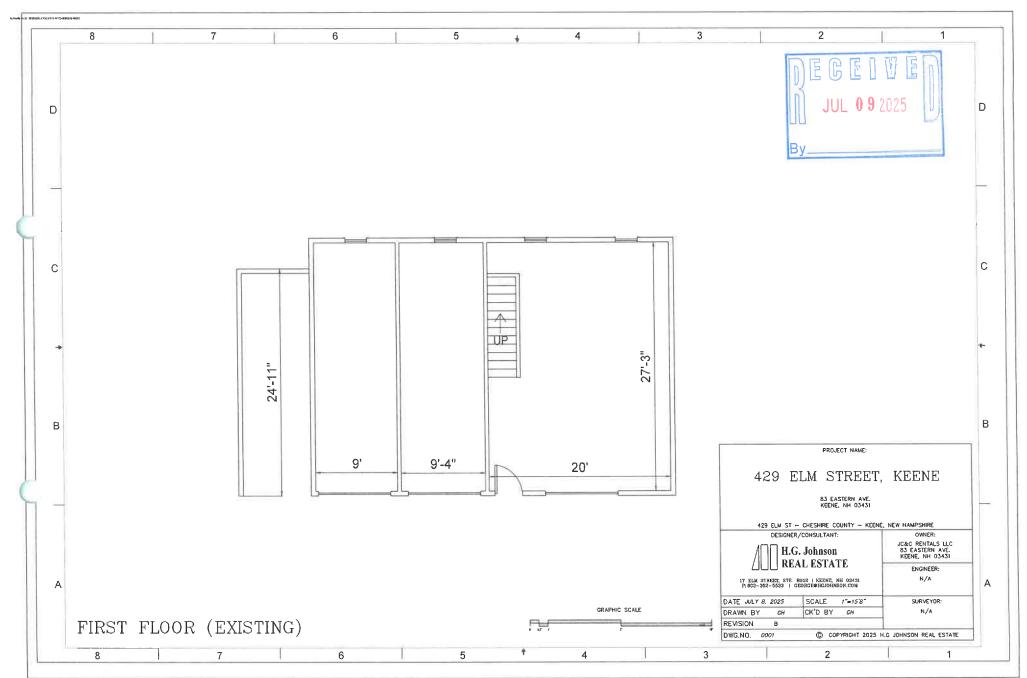
Submitted on behalf of:

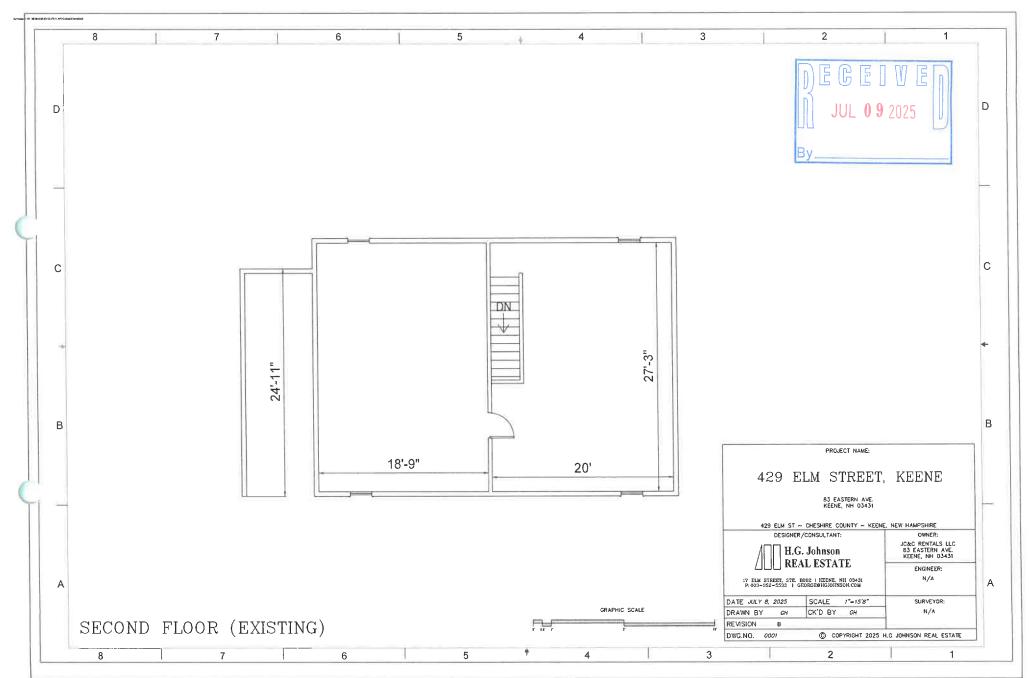
Joe and Cheryl Bagster

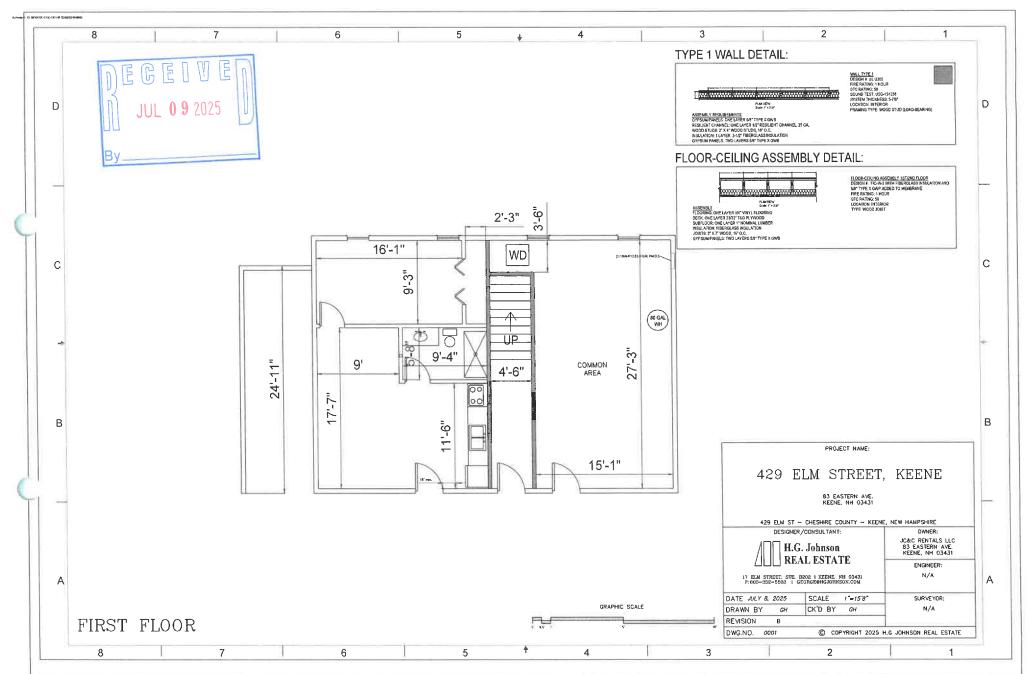
Cheryl & Joe Bagster, Property Owners

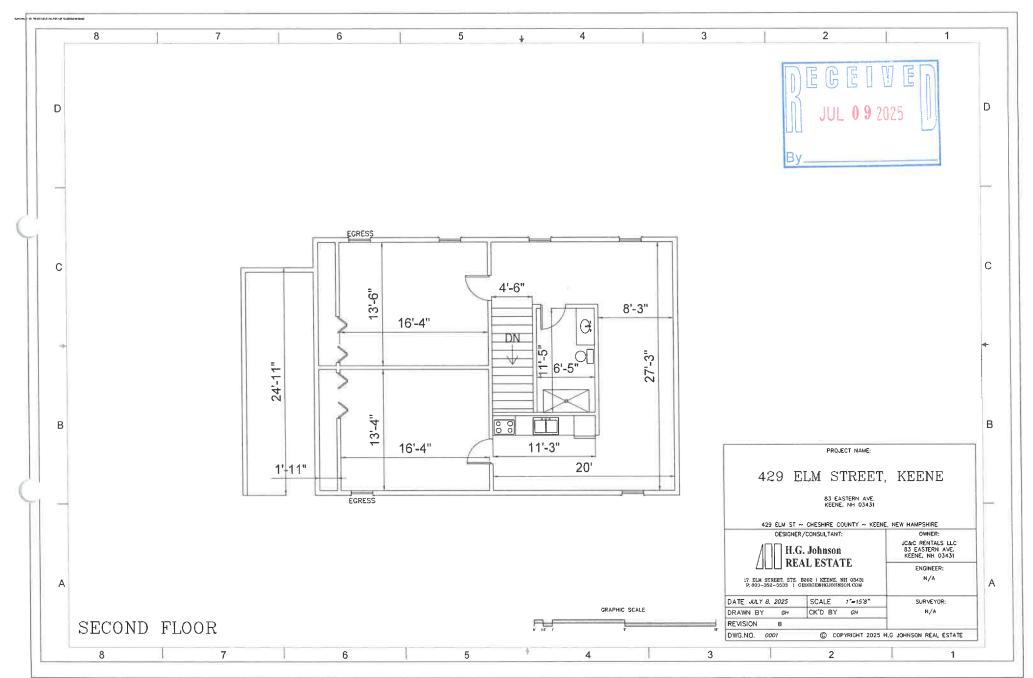
- 3) Manufacturer's cutsheet porch light
- 4) Abutter's List and Mailing Labels













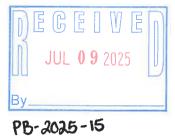
Lighting & Ceiling Fans / Outdoor Lighting / Outdoor Wall Lights

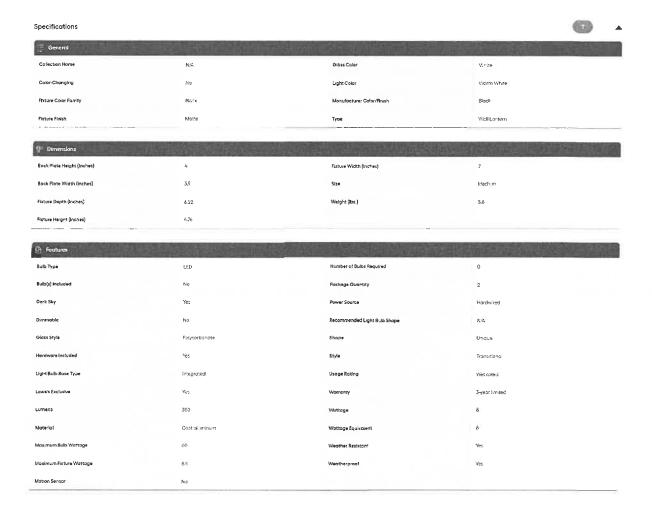


Twin pack black outdoor integrated LED wall dark sky with opal diffuser. Enhances your curb appeal.

- · Cast aluminum construction.
- Black finish with opal PC diffuser.
- 3000K warm white color temperature.
- Suitable in wet location.









MEMORANDUM - AMENDED

TO: Planning Board

FROM: Megan Fortson, Planner

DATE: August 22, 2025

SUBJECT: PB-2024-20 - Earth Excavation Permit Major Amendment & Hillside

Protection Conditional Use Permit - 21 & 57 Route 9

Recommendation:

Planning Staff will provide a recommended motion on the Earth Excavation Permit Major Amendment & Hillside Protection Conditional Use Permit, PB-2024-20, at the Planning Board meeting on August 25, 2025.

Background:

Following the distribution of the previous agenda packet and memo for the August 25th Planning Board meeting, City Staff met with the applicant's team and the Board's third-party consultants to discuss the revised materials that had been submitted along with the review letters drafted by the consultants. During this meeting, all parties came to the consensus that with a few revisions to existing materials and the submittal of additional information, this application would likely be ready for deliberation at the August Planning Board meeting.

On August 21st, the applicant submitted a revised stormwater management report, plan set, a response letter to Fieldstone Land Consultants, and an amended acid mine drainage monitoring plan. Additionally, the results of a "slug test" performed by Front Geoservices on one of the existing bedrock monitoring wells was also submitted. These updated materials were forwarded to the Planning Board's third-party engineering consultant, Fieldstone Land Consultants, as well as their third-party hydrogeology consultant, Sanborn, Head & Associates for review on the same day.

Following their review of the updated materials, Sanborn Head submitted the attached memo dated August 22nd stating that the updated materials adequately addressed their comments related to excavation below the water table, acid mine drainage, and stormwater infiltration. Included in the memo were two recommended conditions of approval related to measuring seasonal high water levels in one of the bedrock monitoring wells and installing an additional well in Permit Period 1. Fieldstone agreed with the feedback provided by Sanborn Head and communicated to staff via email that their comments had also been sufficiently addressed.

Included as attachments are the review letter from Sanborn Head as well as the updated materials submitted by the applicant. Links to the applicable Planning Board agenda packets and meeting minutes are included below. Additional information is available on the Planning Board webpage at https://keenenh.gov/planning-board/.



Planning Board Agenda Packets:

- February 24, 2025
- March 24, 2025
- May 27, 2025
- July 28, 2025

Planning Board Meeting Minutes:

- February 24, 2025
- March 24, 2025
- May 27, 2025



MEMORANDUM

To: Megan Fortson, Planner, City of Keene, NH

From: Russell Abell, PG

File: 6741.00

Date: August 22, 2025

Re: Summary of Hydrogeologic Review of G2 Holdings Keene Quarry Expansion

Application – Second Addendum Materials/Updates

cc: Mari Brunner, Evan Clements, Emily Duseau

Sanborn Head and Associates, Inc. (Sanborn Head) reviewed revised and additional information provided by G2 Holdings (Applicant) in response to our August 15, 2025 Summary of Hydrogeologic Review of G2 Holdings Keene Quarry Expansion Application Summary Memorandum (August 2025 Memo). As requested, this review of revised/additional information focused on reviewing the following:

- 1) Frontier Geoservices, GORDON SERVICES KEENE PIT BRW-12 SLUG TEST DATA & RESULTS, 57 Route 9, Keene, New Hampshire, August 21, 2025.
- 2) Frontier Geoservices, GORDON SERVICES KEENE PIT ACID MINE DRAINAGE MONITORING PROGRAM, 57 Route 9, Keene, New Hampshire, August 21, 2025.
- 3) Granite Engineering, LLC, 2025, Gravel And Earth Removal Plan G2 Holdings LLC, Keene Tax Map 215 Lots 7 & 8, Sullivan Tax Map 5 Lots 46 & 46-1, 57 Route 9, Keene New Hampshire, Cheshire County, Revised August 21.

The following provides the concerns summarized in our August 2025 Memo and Sanborn Head's responses based on review of the information reviewed in items 1-3 above and information stated during an August 18, 2025 meeting (August 2025 Meeting) with the Applicant and their technical/legal team. Each concern from the August 2025 Memo is provided below in italicized text followed by Sanborn Head's response in plain text font.

1. Excavation Below the Water Table: The observed water table in newly installed monitoring wells located in proposed excavation area Period 8 represents water table conditions that can be characterized as seasonal low water table conditions. Therefore, the revised Period 8 "Pit Floor" proposed elevation of 860 feet above mean sea level (AMSL) has the potential to be below the water table and/or lower than six feet above the water table during seasonal high water table conditions. This is especially of concern in the northwest portion of the proposed excavation area. In fact, as shown on Figure 2, under current seasonal low water table conditions, the northwest portion of the Period 8 excavation is projected to be below the water table. Seasonal high water conditions may also be a concern for Period 1 as well, especially in the southeastern portion of the

excavation area. Additional data collection from Period 1 would be needed to determine the water table conditions there.

As requested and agreed upon during the August 2025 Meeting, the Applicant has provided a response to the above concern. The applicant stated with respect to Period 8 that they had revised the final proposed grade elevations for the bottom of the excavation in the northern portion of Period 8 to account for water level elevations in that area and the provided site plans appear to confirm these changes. Discussion during the August 2025 meeting and the revised grade elevations in Period 8 satisfy the above concern as long as a condition of the permit requires seasonal high (e.g., spring conditions) water level monitoring in BRW-09 to confirm the revised grades will be in compliance with the 6 feet above the water table requirement in City of Keene Land Development Code (LDC) Article 25.3.3.

For Period 1, as discussed and agreed upon during the August 2025 meeting, a condition of the permit is recommended to install a monitoring well within Period 1 to confirm the seasonal high water table elevation conditions (e.g., spring conditions) in this area as current data are not sufficient. This is needed to confirm that proposed grade elevations of the base of the Period 1 excavation are in compliance with LDC Article 25.3.3. Addition of this condition will satisfy the above concern with respect to the water table elevation in Period 1.

2. **AMD Potential:** The Application supporting documentation has presented a concern for [acid mine drainage] AMD potential as a result of excavation of bedrock in each of the proposed Periods. The recently observed presence of the mineral pyrite (an iron sulfide mineral with chemical formula FeS₂) in drill cuttings generated during the installation of monitoring wells in Period 8 further supports this concern. Although the available information does not allow for a more thorough assessment of AMD development potential, several existing lines of evidence also point to it being a concern. While the Application has provided a monitoring program, best management practice of lining surface water detention and retention ponds with limestone aggregate, and excavation will be terminated if necessary, additional monitoring should be considered and appears warranted.

As requested and agreed upon during the August 2025 Meeting, the Applicant has provided a revised Acid Mine Drainage Monitoring Plan that includes the additional requested items. These items include:

- installation and monitoring of an additional bedrock monitoring well downgradient (north) of Period 8;
- quarterly monitoring for a year for each monitoring location with a change to semi-annual if results do not indicate a concern for AMD for each excavation area (Period);
- clarification that stormwater basins will be part of the monitoring program as they are constructed; and



 field screening of pH will occur monthly in the active stormwater basins (at the time) and "BRW" series monitoring wells that are identified as part of the program.

The modified monitoring plan satisfies this concern.

3. **Period 8 Stormwater Infiltration:** Although not a focus of this review, another concern identified relates to the stormwater management plan for the proposed project. Based on our preliminary review and understanding, an existing infiltration basin (retention basin) and a newly constructed basin in Period 8 will serve to infiltrate all stormwater that emanates at the Site during the implementation of the project. The documentation appears to show the newly constructed Period 8 retention basin will be constructed on the bedrock pit "floor" (or will be excavated into bedrock), and will exist as the only infiltration basin during the late stages of the project (excavation of Period 8), if it is not also active earlier in the project. With the base located on bedrock, even if saprolite or highly weathered (typically clay-rich) materials are present, it seems unlikely that an infiltration rate of 0.3 inches per hour will be achieved as the Application used to determine stormwater management. It is possible that the actual infiltration rate could be close to zero (if competent, unfractured bedrock), or as much as an order of magnitude lower (if weathered bedrock or saprolite). Therefore, it appears unlikely that stormwater will infiltrate in this Period 8 basin, especially at the rate used to model stormwater management.

As discussed and requested during the August 2025 Meeting, the Applicant has also responded to this concern with information stated in the meeting and an additional field test completed at the site. The Applicant provided the results of an in-situ hydraulic conductivity test, which measures the capacity of the subsurface materials to transmit water. Sanborn Head has reviewed this information and analysis, which indicates that the underlying material at the approximate elevation of the planned Period 8 stormwater infiltration basin has a similar hydraulic conductivity to a silty sand. The estimated infiltration rate used in the Applicant's stormwater modeling is for a similar material and estimated at 0.3 inches per hour. Based on the following, this concern is satisfied by the additional information provided:

- The hydraulic conductivity testing results in BRW-12 indicates a conductivity similar to a silty sand material, which is similar to the estimated infiltration rate of the material used in the stormwater model;
- The Applicant's technical team stated that during the drilling of monitoring wells in Period 8 competent bedrock was not observed;
- The Applicant's technical team stated that monitoring wells in Period 8
 recharged quickly after removal of groundwater for sampling (purging) and well
 development, which is consistent with the conductivity testing results above;
 and
- The Applicant's technical team stated that the current infiltration basin located in Period 8 is in the same material (albeit at a higher elevation), and they have

observed infiltration occurring quickly after significant rain events (e.g., the basin mostly drained within 24 hours after filling during a rain event.)

Based on the above, this concern is satisfied.

At this time, as long as the two recommended conditions are included in the permit, Sanborn Head does not have additional concerns with the proposed excavation permit plans with respect to the two areas we were retained to review: LDC Article 25.3.3.B and LDC Article 25.3.6 waivers. Also, concerns expressed previously regarding the infiltration basin in Period 8 have been satisfied and Sanborn Head does not have further concerns for this item.

We trust this information meets your needs at this time. Please contact me should you have questions regarding this information. We appreciate the opportunity to support the Town's review of the Application.

RHA/SRN: rha



August 21, 2025

City of Keene Community Development Department – Planning and Zoning 3 Washington Street Keene, New Hampshire 03431

RE: G2 Holdings, LLC Tax Map 215 Lots 7 & 8 21 & 57 Route 9, Keene, NH GE Project No. 2302011

Dear Ms. Fortson,

We are in receipt of a consultant review report, recieved August 14, 2025, relative to the review of the Earth Excavation Permit application, PB-2024-20, for the G2 Holdings, LLC project located at 21 & 57 Route 9. In addition to responses to your comments, please find the following material in support of the referenced project:

- Three (3) Copies of the revised plans (22" x 34")
- Three (3) Copies of the Stormwater Management Report
- Digital submission of the updated materials

In response to the comments made by Fieldstone Land Consultants, PLLC, we offer the following explanations and/or responses:

Section 25 Earth Excavation Permit:

4. Section 25.3.3: The third-party hydrogeologist (Sanborn Head) observed the borings and monitoring wells and has represented that there are potential conflicts with the groundwater and/or separation to groundwater in Period 1 and Period 8. This should be reviewed and the design and stormwater management report revised as necessary.

The grading for periods 1 and 8 have been adjusted to maintain a minimum of 6' above water table elevations observed. Per the 8/20/2025 meeting, a condition to the permit will require a monitoring well to be installed within period 1 during spring. If the water table is observed within 6' of the proposed grading, the grading will be slightly raised to maintain the required 6' separation.

The stormwater report states that the design is relying on test pits performed by TFMoran but those test pits do not reach depths to verify soil materials and adequate separation to seasonal high-water table. The Subsurface Data table on the monitoring plan shows that Test Pit 1 and 2 go to depths of 867.0 and 863 respectfully and the infiltration basin in this area has a bottom elevation of 842.0.

The stormwater report <u>narrative</u> has been revised to reference the newly drilled well bedrock wells, including BRW-12, which observed water at elevation 830.92. The colored drainage plans have also been revised to show the revised pit grading for periods 1 and 8.

SLR-10 which is outside of the infiltration area shows water observed at 841.1 (884-42.9) which would not provide adequate separation to the bottom of the infiltration basin at 842.0. The ground elevation per the topography depicted on the monitoring plan shows SLR-10 at elevation 890+/- not 884 per the table. If the elevation in the plan view is correct this would raise the observed groundwater level to 847.1+/-. BRW-12 which is inside the infiltration basin is not detailed on the monitoring plan but it is my understanding readings from BRW-12 may support. ground water elevations at approximately 830.9 during low flow conditions but the materials are not conducive to infiltration since bedrock was encountered. The table on the monitoring plan should be updated to depict all observations on-site and the data should be verified to ensure that all elevations are represented correctly.

SLR 10 was an existing well that was installed during the previous project designed and permitted by another firm. It no longer exists on site due to excavation activities in that area. All references to SLR wells done under the previous project have been removed from the planset, as they are no longer present on site. BRW 12 was shown on the monitoring plan sheet 17 of 23 in lieu of SLR 10. This well was dug to replace SLR 10.

As discussed in the 8/20/25 meeting, a hydraulic conductivity slug test was performed at BRW 12 and the data support the infiltration used in the stormwater design. The conversation revolved around the presence of ledge near the infiltration pond which was explained by our team to not be the case. It was agreed that this material was not typical ledge and the infiltration test provided that.

6. Section 25.3.4.A.2: The water table drops 22+/- feet between SLR10 and SLR11 and it is a relatively short distance between these two locations. We would recommend an additional test site between the two locations to ensure adequate separation to seasonal high water. This stormwater management area is critical to the design and operation of this site. This additional testing could be done between phases as a condition of approval should the Board feel comfortable with this recommendation.

BRW-12 has been installed between former SLR10 and SLR11 and within the floor of the infiltration pond. The water table was observed at 830.92, and the proposed pit floor is at elevation 840.00.

1. New Comments: Will on-site vibration monitoring be done during blasting and crushing operations? Concerns have been raised regarding adjacent properties and potential damage nearby infrastructure associated with on-site operations. This type of monitoring is typically required to protect adjacent properties and to address liability issues in this industry. Please share how the site operations will address this concern and be consistent with city regulations. We would recommend adding a note to the plan set to document how the site operations will comply with city and industry standards.

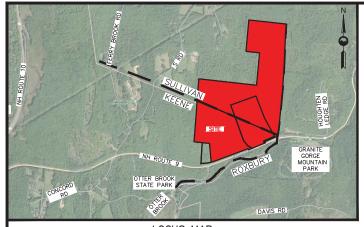
General Note 21 regarding vibration monitoring has been added on sheet 1.

We trust the noted plan revisions and/or explanations will adequately address the conditions listed above. Should you have any further questions or comments, please do not he sitate to contact this office.

Best Regards,

Justin Daigneault

Project Manager

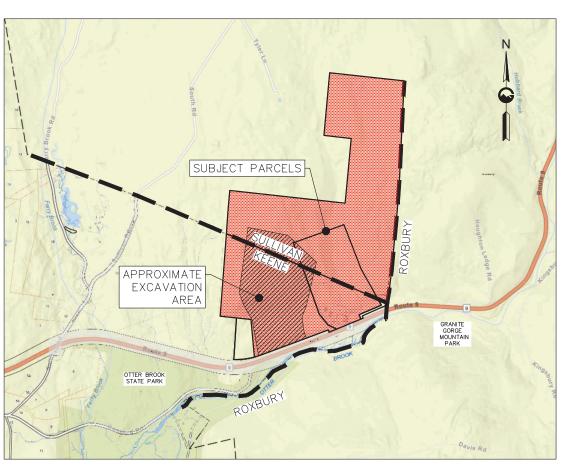


LOCUS MAP SCALE: ±1"=2,000'

GRAVEL AND EARTH REMOVAL PLAN

G2 HOLDINGS, LLC

KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE, NEW HAMPSHIRE
CHESHIRE COUNTY















OWNER & APPLICANT:

G2 HOLDINGS, LLC 250 NORTH STREET JAFFREY, NH 03452 (603) 325-8457

CIVIL ENGINEER:

GRANITE ENGINEERING, LLC 150 DOW STREET, TOWER 2, STE 421 MANCHESTER, NH 03101 (603) 518-8030

WETLAND SCIENTIST:

ECOSYSTEMS LAND PLANNING 36 DUNKLEE STREET CONCORD, NH 03301 (603) 224-6244

SURVEYOR:

SMITH & POSPESIL LAND SURVEYING, PLLC 240 QUEBEC ROAD LYMAN, NH 03585 (603) 838-6494

SOIL SCIENTIST:

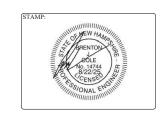
HURLEY ENVIRONMENTAL AND LAND PLANNING, LLC.
P.O. BOX 356
EPSOM, NH 03234
(603) 583-1745

HYDROGEOLOGIST:

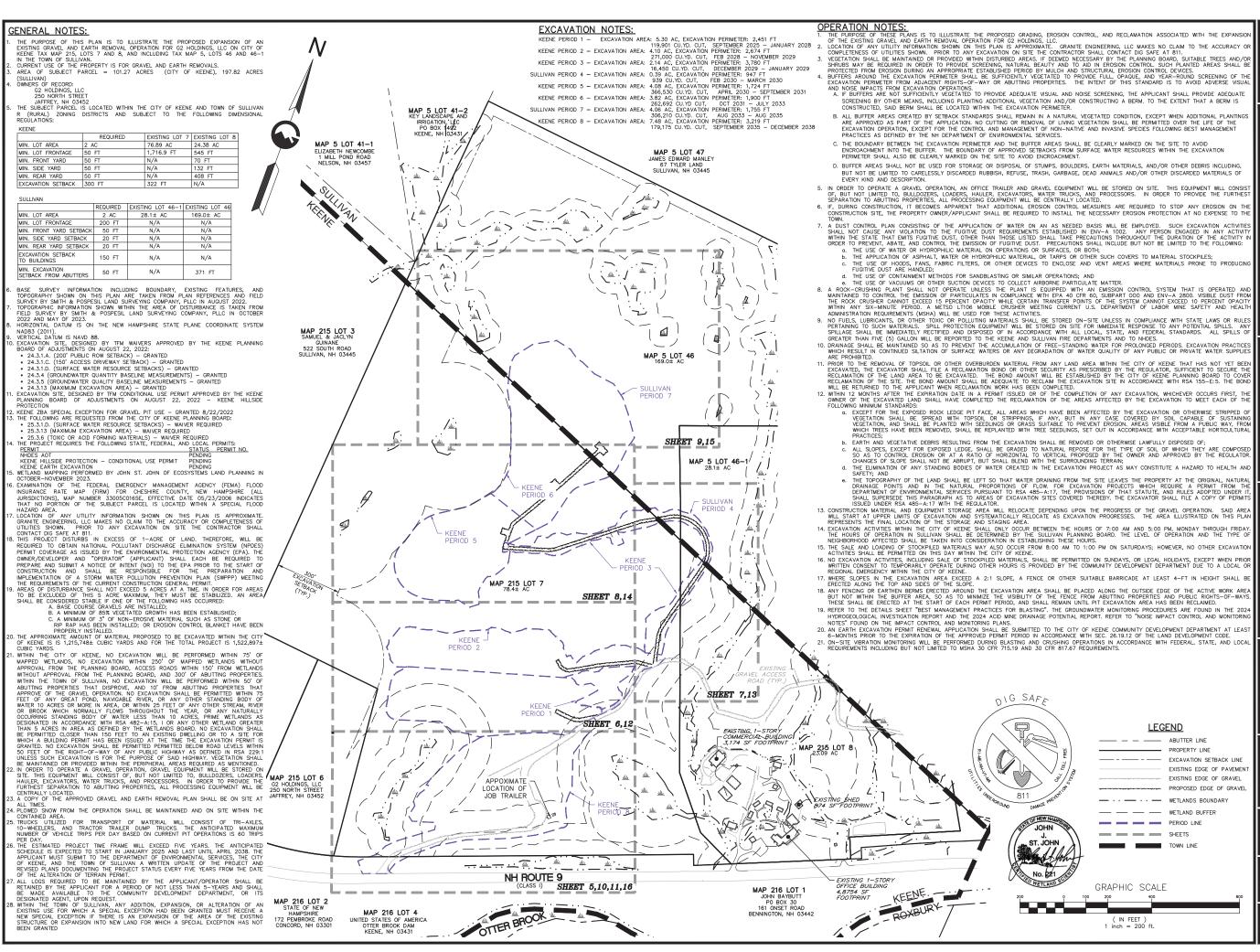
FRONTIER GEOSERVICES, LLC. 127 OLD WARNER ROAD BRADFORD, NH 03221 (603) 748-37155

SHEET NO.	TABLE OF CONTENTS	
1	OVERVIEW PLAN	
2-3	EXISTING CONDITIONS PLAN WITH BOUNDARY LINES	
4	CONTEXT PLAN	
5-10	EXCAVATION, DRAINAGE & EROSION CONTROL PLAN	
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17	MONITORING PLAN	
18-19	RECLAMATION PLAN	
20-23	DETAILS	





		REVISIONS	
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1	12/20/24	PROJECT SUBMITTAL	JD
2	2/3/25	REVISED PER CITY COMMENTS	JD
3	5/9/25	REVISED PER CITY COMMENTS	JD
4	7/9/25	REVISED PER CITY COMMENTS	JD
5	7/24/25	ADDITIONAL WELL LOCATIONS	JD
6	8/11/25	REVISED PER CITY COMMENTS	JD
7	8/22/25	REVISED PER CITY COMMENTS	JD
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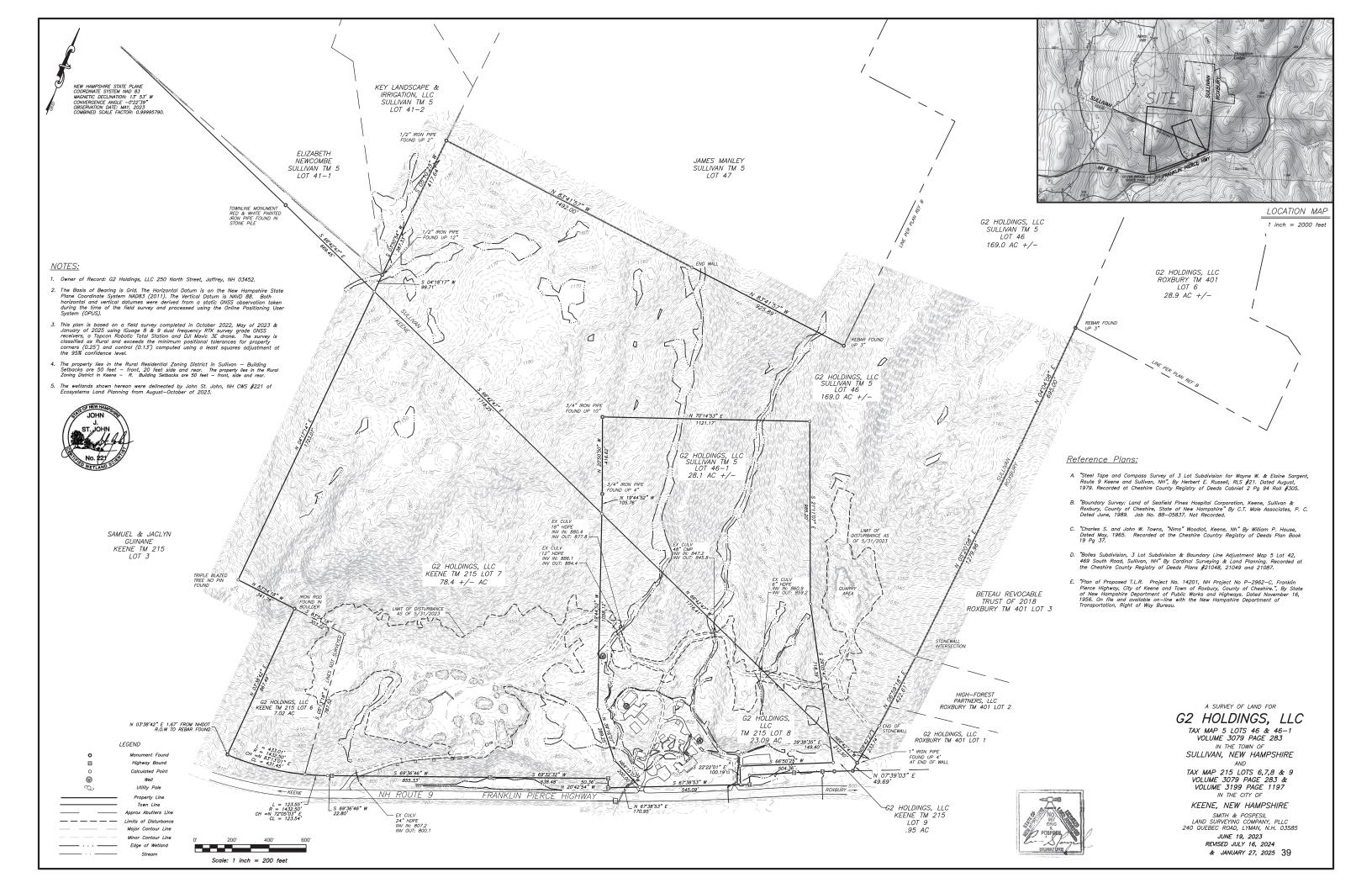
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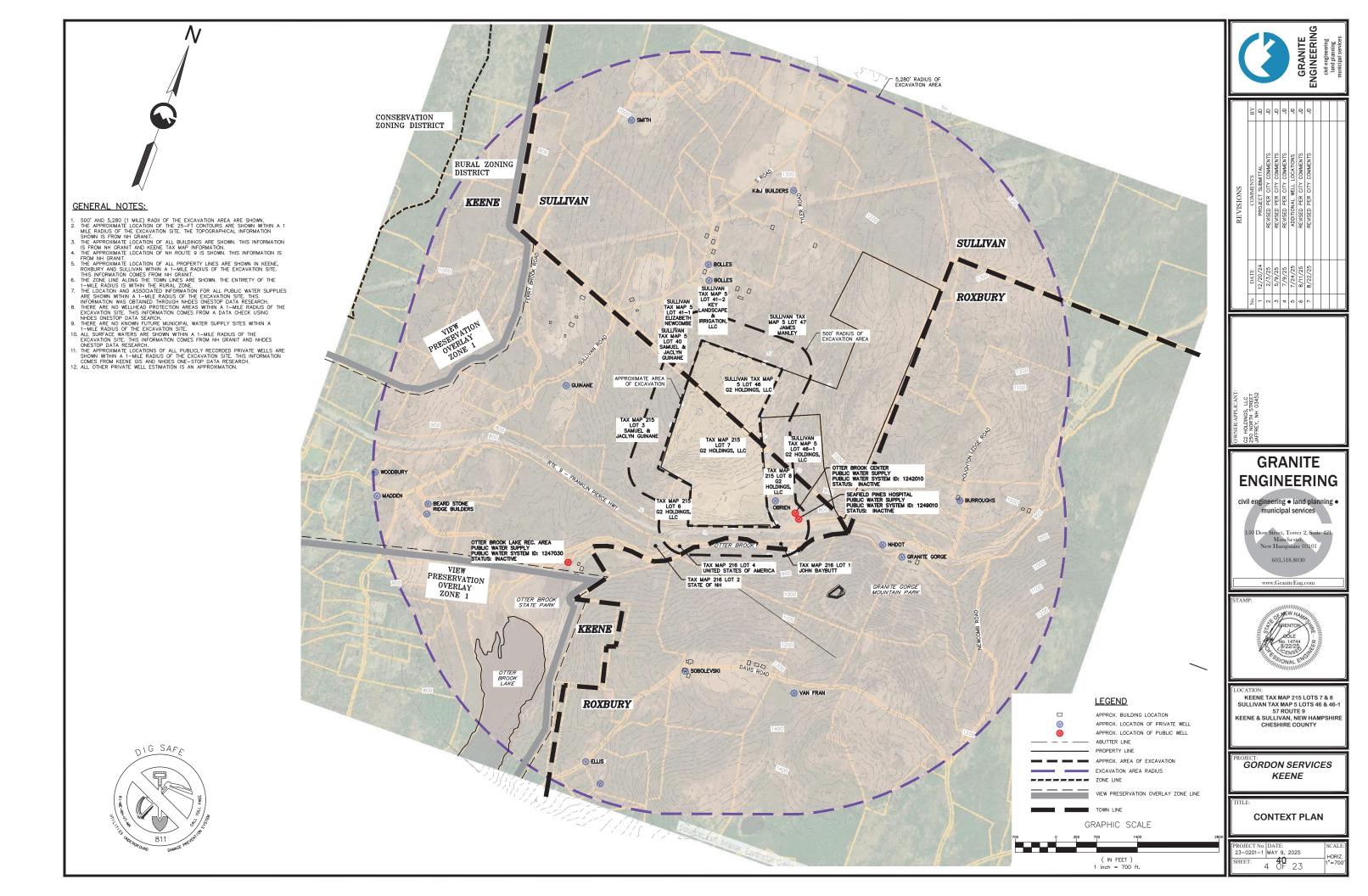
GORDON SERVICES KEENE

OVERVIEW PLAN

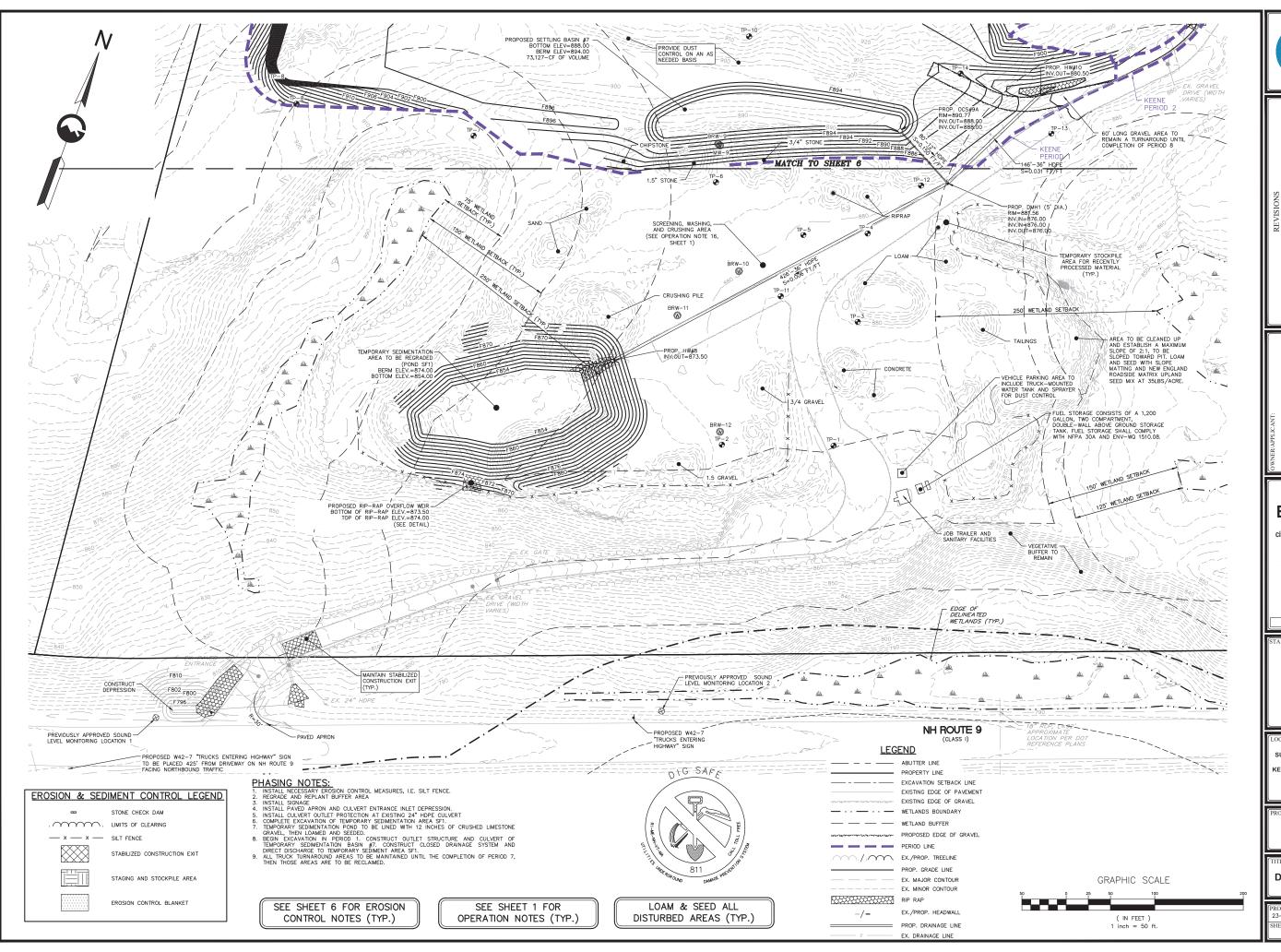
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OWNER APPLICANT G2 HOLDINGS, LLC 250 NORTH STREET JAFFREY, NH 03452

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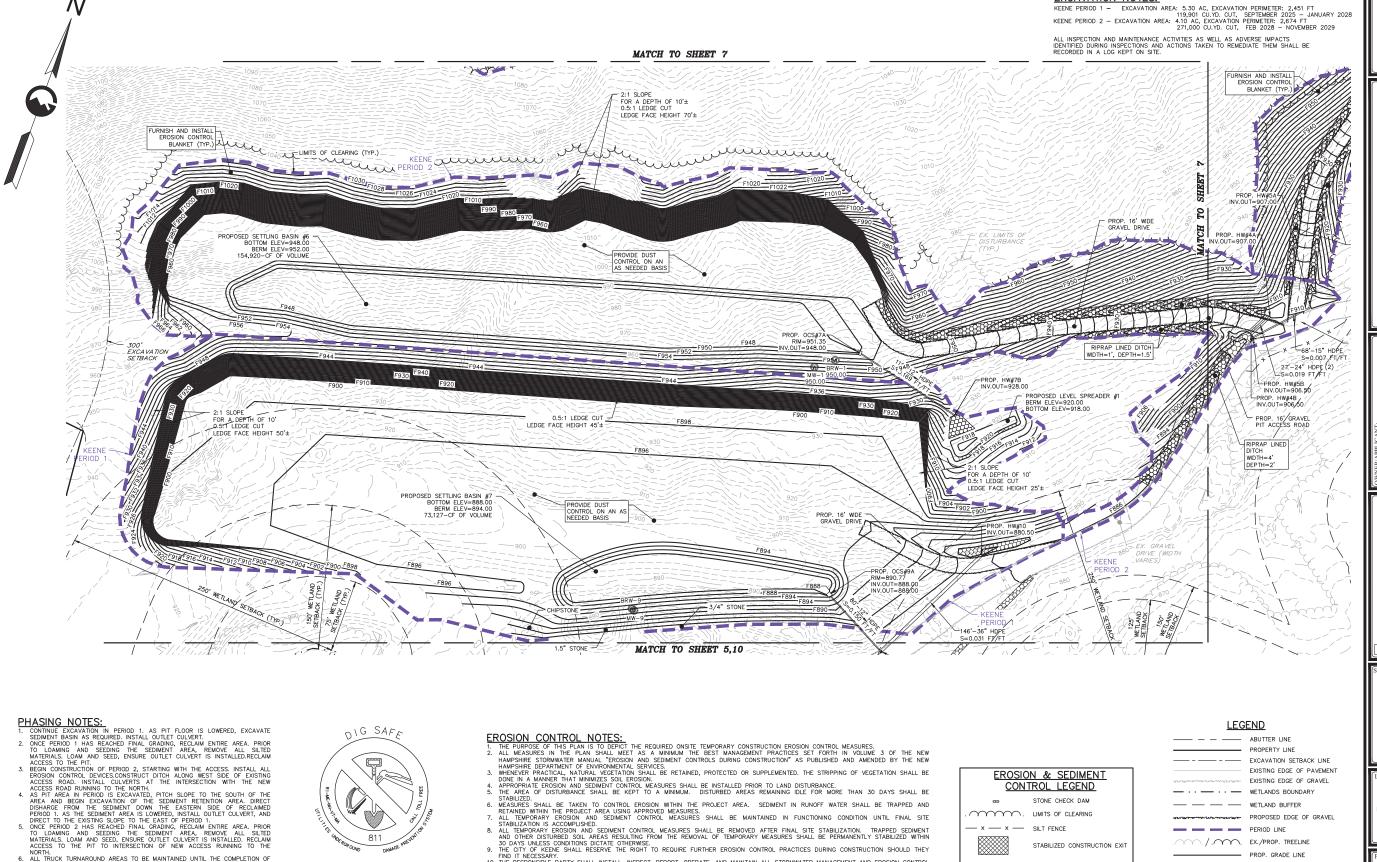
LOCATION:

KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

OJECT:
GORDON SERVICES
KEENE

DRAINAGE & EROSION
CONTROL PLAN

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 HORIZ, SHEET: 5 OF 23 12=50



NORTH. ALL TRUCK TURNAROUND AREAS TO BE MAINTAINED UNTIL THE COMPLETION OF PERIOD 7, THEN THOSE AREAS ARE TO BE RECLAIMED.



SEE SHEET 1 FOR OPERATION NOTES (TYP.)

LOAM & SEED ALL DISTURBED AREAS (TYP.)

4. APPROPRIATE EROSION AND SEDMENT OF VINCU MEASURES SHALL BE INSTALLED AREAS REMAINING IDLE FOR MORE THAN 30 DAYS SHALL BE TARRED OF DISTURBANCE SHALL BE KEPT TO A MINIMUM. DISTURBED AREAS REMAINING IDLE FOR MORE THAN 30 DAYS SHALL BE MEASURES SHALL BE TAKEN TO CONTROL EROSION WITHIN THE PROJECT AREA. SEDIMENT IN RUNOFF WATER SHALL BE TRAPPED AND RETAINED WITHIN THE PROJECT AREA USING APPROVED MEASURES.

7. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED IN FUNCTIONING CONDITION UNTIL FINAL SITE STABILIZATION IS ACCOMPLISHED.

8. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED AFTER FINAL SITE STABILIZATION. TRAPPED SEDIMENT AND OTHER DISTURBED SOIL AREAS RESULTING FROM THE REMOVAL OF TEMPORARY MEASURES SHALL BE PERMANENTLY STABILIZED WITHIN 30 DAYS UNLESS CONDITIONS DICTATE OTHERWISE.

7. THE CITY OF KEENE SHALL RESERVE THE RIGHT TO REQUIRE FURTHER EROSION CONTROL PRACTICES DURING CONSTRUCTION SHOULD THEY FIND IT NECESSARY.

8. THE CITY OF KEENE SHALL RESERVE THE RIGHT TO REQUIRE FURTHER EROSION CONTROL PRACTICES DURING CONSTRUCTION SHOULD THEY FIND IT NECESSARY.

8. THE CITY OF KEENE SHALL BE INSTALLED IN STRICT ACCORDANCE WITH PROJECT PLANS. IN ADDITION, SIMILAR MEASURES SHALL BE INSTALLED WHERE AND WHEN THE FIELD CONDITION, OR FIELD OPERATION OF THE INDIVIDUAL SITE CONTRACTOR, MAY WARRANT.

12. ALL DISTURBED AREAS DESIGNATED TO BE TURFE. SHALL BE RECEIVE A MINIMUM APPLICATION OF THE INDIVIDUAL SITE CONTRACTOR, MAY WARRANT.

WARRANT.

12. ALL DISTURBED AREAS DESIGNATED TO BE TURF, SHALL RECEIVE A MINIMUM APPLICATION OF 4 INCHES OF LOAM (COMPACTED THICKNESS), PRIOR TO FINAL SEEDING AND MULCHING, 13. IN THE EVENT THAT, DURING CONSTRUCTION OF ANY PORTION OF THIS PROJECT, A WINTER SHUTDOWN IS NECESSARY, THE CONTRACTOR SHALL STABILIZE ALL INCOMPLETE WORK AND PROVIDE FOR SUITABLE METHODS OF DIVERTING RUNOFF IN ORDER TO ELIMINATE SHEET FLOW ACROSS FROZEN SURFACES.

14. DUST SHALL BE CONTROLLED BY THE USE OF WATER AS NECESSARY THROUGHOUT THE CONSTRUCTION PERIOD, IN ACCORDANCE WITH ENVY ALD THOSE TRADES TO THE OWNER OF THE PROPERTY OF THE PROPE

1000.

15. IN NO WAY ARE THOSE TEMPORARY EROSION CONTROL MEASURES INDICATED ON THESE PLANS TO BE CONSIDERED ALL INCLUSIVE. THE
CONTRACTOR SHALL USE JUDGEMENT IN INSTALLING SUPPLEMENTARY EROSION CONTROL MEASURES WHERE AND WHEN SPECIFIC SITE

CONTRACTOR SHALL USE DUBLEMENT IN INSTALLING SOFTEMENTARY ENGINEERS SHALL BE CONSTRUCTION METHODOLOGIS MAY WARRANT.

16. GRADED AREAS SHALL BE VEGETATED TO INSURE EROSION CONTROL BY SEEDING, MULCHING, AND FERTILIZING. DISTURBED AREAS SHALL BE PLANTIED WITH SUTTABLE PLANT MATERIALS.

17. GRADING SHALL NOT EXCELD A RATIO OF A HORIZONTAL TO I VERTICAL WITHOUT SPECIAL EROSION CONTROL MEASURES. NETTING OR SIMILAR MATERIAL SHALL BE FROVIDED ON SLOPES WITH A RATIO GREATER THAN 3:1 WHILE GROUND COVER IS BEING ESTABLISHED.

. LIMITS OF CLEARING PROPOSED EDGE OF GRAVEL PERIOD LINE STABILIZED CONSTRUCTION EXIT PROP. GRADE LINE EX. MAJOR CONTOUR STAGING AND STOCKPILE AREA EX. MINOR CONTOUR

EXCAVATION NOTES:

RIP RAP EROSION CONTROL BLANKET

PROP. DRAINAGE LINE

EX./PROP. HEADWALL

GRAPHIC SCALE





		REVISIONS	
No.	DATE	COMMENTS	BY
-	12/20/24	PROJECT SUBMITTAL	9
2	2/3/25	REVISED PER CITY COMMENTS	9
23	5/9/25	REVISED PER CITY COMMENTS	9
4	7/9/25	REVISED PER CITY COMMENTS	as G
2	7/24/25	ADDITIONAL WELL LOCATIONS	9
9	8/11/25	REVISED PER CITY COMMENTS	9
7	8/22/25	REVISED PER CITY COMMENTS	9

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KEENE TAX MAP 215 LOTS 7 & 8 SULLIVAN TAX MAP 5 LOTS 46 & 46-1 57 ROUTE 9 KEENE & SULLIVAN, NEW HAMPSHIRE

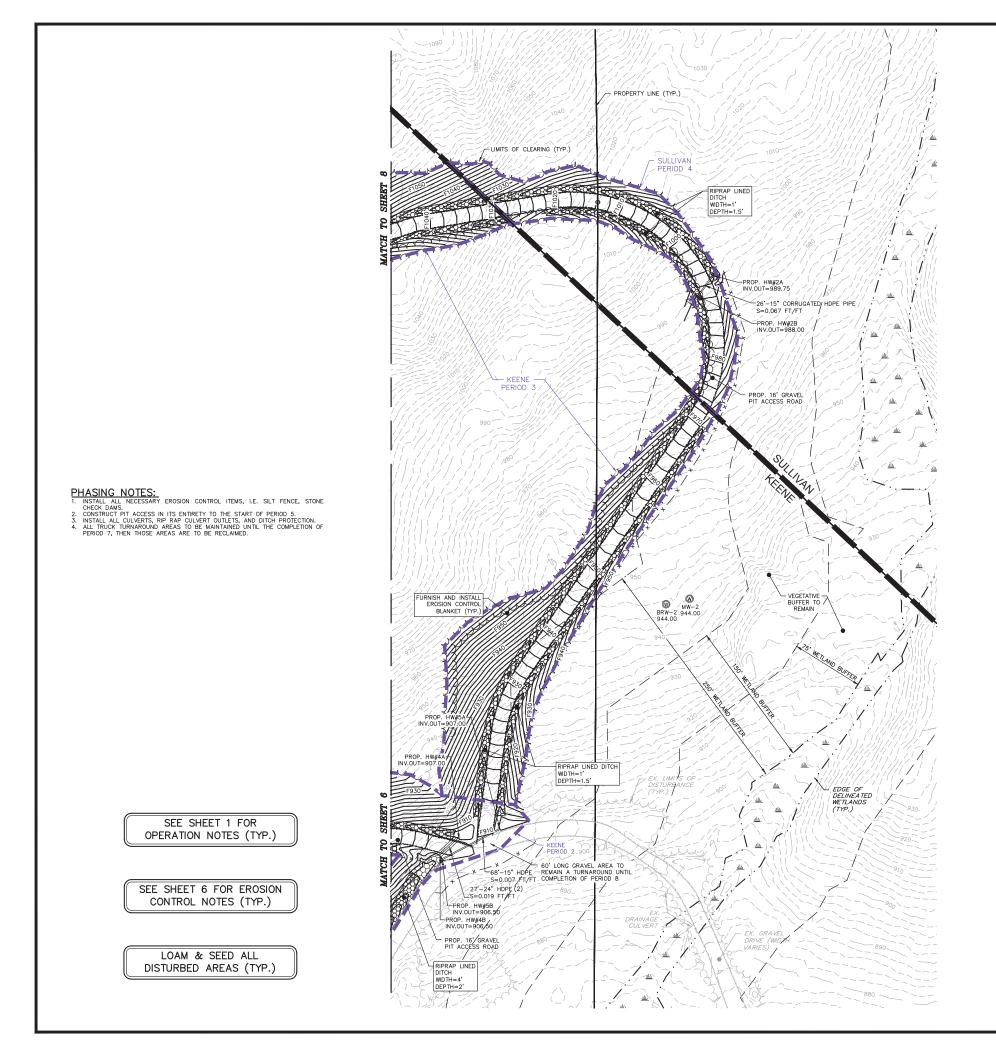
GORDON SERVICES

KEENE

CHESHIRE COUNTY

EXCAVATION, **DRAINAGE & EROSION CONTROL PLAN**

23-0201-1 MAY 9, 2025 6 OF 23



EXCAVATION NOTES:

KEENE PERIOD 3 - EXCAVATION AREA: 2.14 AC, EXCAVATION PERIMETER: 3,780 FT

16,450 CU.YD. CUT, DECEMBER 2029 - JANUARY 2030

SULLIVAN PERIOD 4 - EXCAVATION AREA: 0.39 AC, EXCAVATION PERIMETER: 947 FT

939 CU.YD. CUT, FEB 2030 - WARCH 2030

ALL INSPECTION AND MAINTENANCE ACTIVITIES AS WELL AS ADVERSE IMPACTS IDENTIFIED DURING INSPECTIONS AND ACTIONS TAKEN TO REMEDIATE THEM SHALL BE RECORDED IN A LOG KEPT ON SITE.



LEGEND

ABUTTER LINE
PROPERTY LINE
EXCAVATION SETBACK LINE
EXISTING EDGE OF PAVEMEN
EXISTING EDGE OF GRAVEL
WETLANDS BOUNDARY
WETLAND BUFFER
PROPOSED EDGE OF GRAVE
PERIOD LINE
EX./PROP. TREELINE
PROP. GRADE LINE
EX. MAJOR CONTOUR
EX. MINOR CONTOUR
RIP RAP
EX./PROP. HEADWALL
PROP. DRAINAGE LINE

EROSION & SEDIMENT CONTROL LEGEND

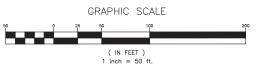
EX. DRAINAGE LINE

STONE CHECK DAM . . LIMITS OF CLEARING

STABILIZED CONSTRUCTION EXIT

STAGING AND STOCKPILE AREA EROSION CONTROL BLANKET







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COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
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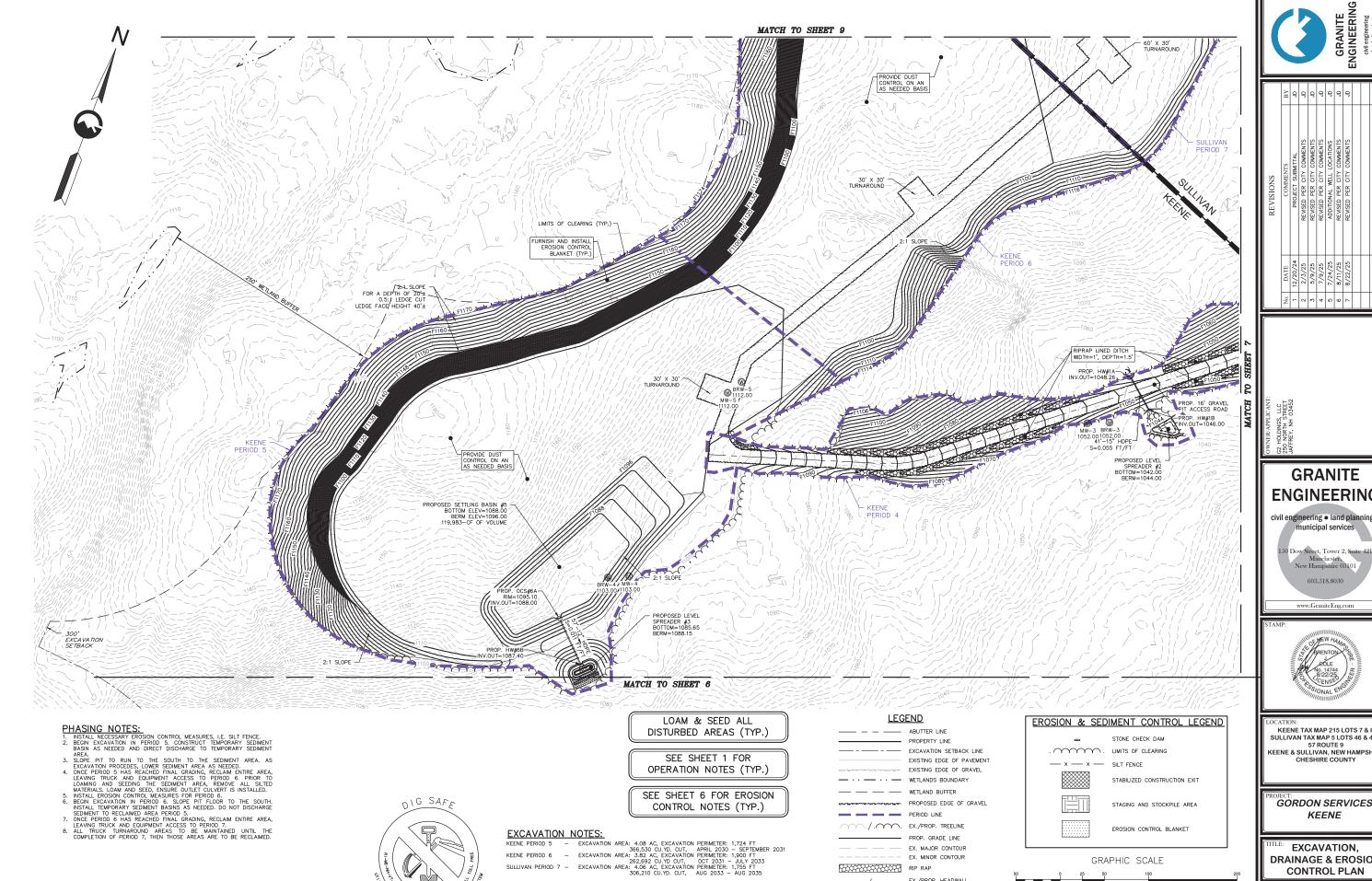


KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

GORDON SERVICES KEENE

EXCAVATION, **DRAINAGE & EROSION** CONTROL PLAN

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 7 OF 23



ALL INSPECTION AND MAINTENANCE ACTIVITIES AS WELL AS ADVERSE IMPACTS IDENTIFIED DURING INSPECTIONS AND ACTIONS TAKEN TO REMEDIATE THEM SHALL BE RECORDED IN A LOG KEPT ON SITE.

EX./PROP. HEADWALL

= PROP. DRAINAGE LINE

(IN FEET)

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REVISIONS	COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
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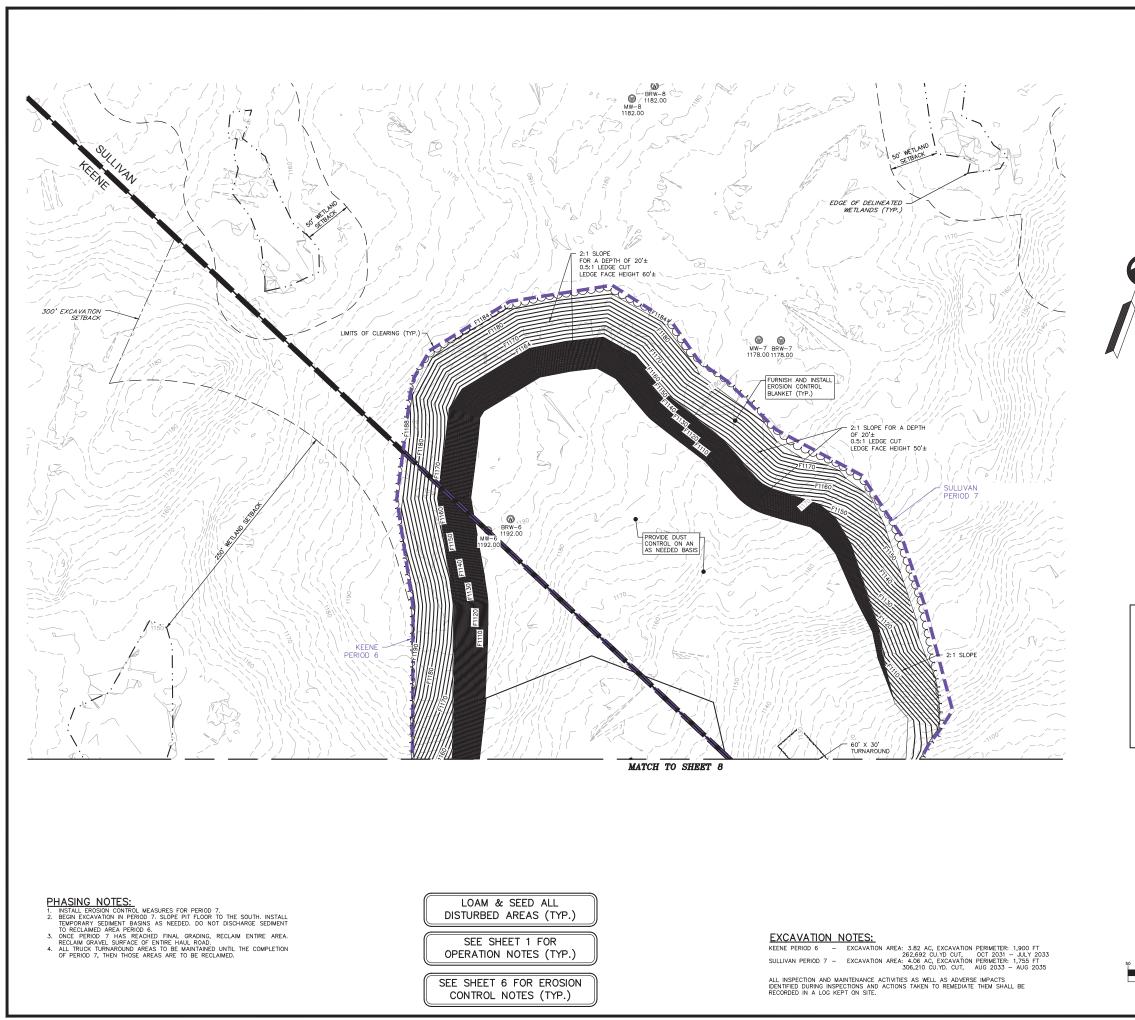
KEENE TAX MAP 215 LOTS 7 & 8

SULLIVAN TAX MAP 5 LOTS 46 & 46-1 57 ROUTE 9 KEENE & SULLIVAN, NEW HAMPSHIRE **CHESHIRE COUNTY**

GORDON SERVICES KEENE

EXCAVATION, **DRAINAGE & EROSION CONTROL PLAN**

23-0201-1 MAY 9, 2025 8 OF 23





LEGEND

____ _ _ _ ABUTTER LINE - PROPERTY LINE ------ EXCAVATION SETBACK LINE EXISTING EDGE OF PAVEMENT EXISTING EDGE OF GRAVEL - · · - · · - WETLANDS BOUNDARY PROPOSED EDGE OF GRAVEL

. EX./PROP. TREELINE

EX. MINOR CONTOUR RIP RAP

EX./PROP. HEADWALL -/**-**

EROSION & SEDIMENT CONTROL LEGEND

STONE CHECK DAM . . LIMITS OF CLEARING - SILT FENCE

STABILIZED CONSTRUCTION EXIT

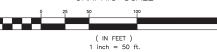
PROP. DRAINAGE LINE

STAGING AND STOCKPILE AREA

EROSION CONTROL BLANKET

DIG SAFF

GRAPHIC SCALE



EXCAVATION, **DRAINAGE & EROSION** CONTROL PLAN

PROJECT No. DATE:	SCALE:
23-0201-1 MAY 9, 2025	HORIZ.
SHEET: 45	1"=50"
9 OF 23	

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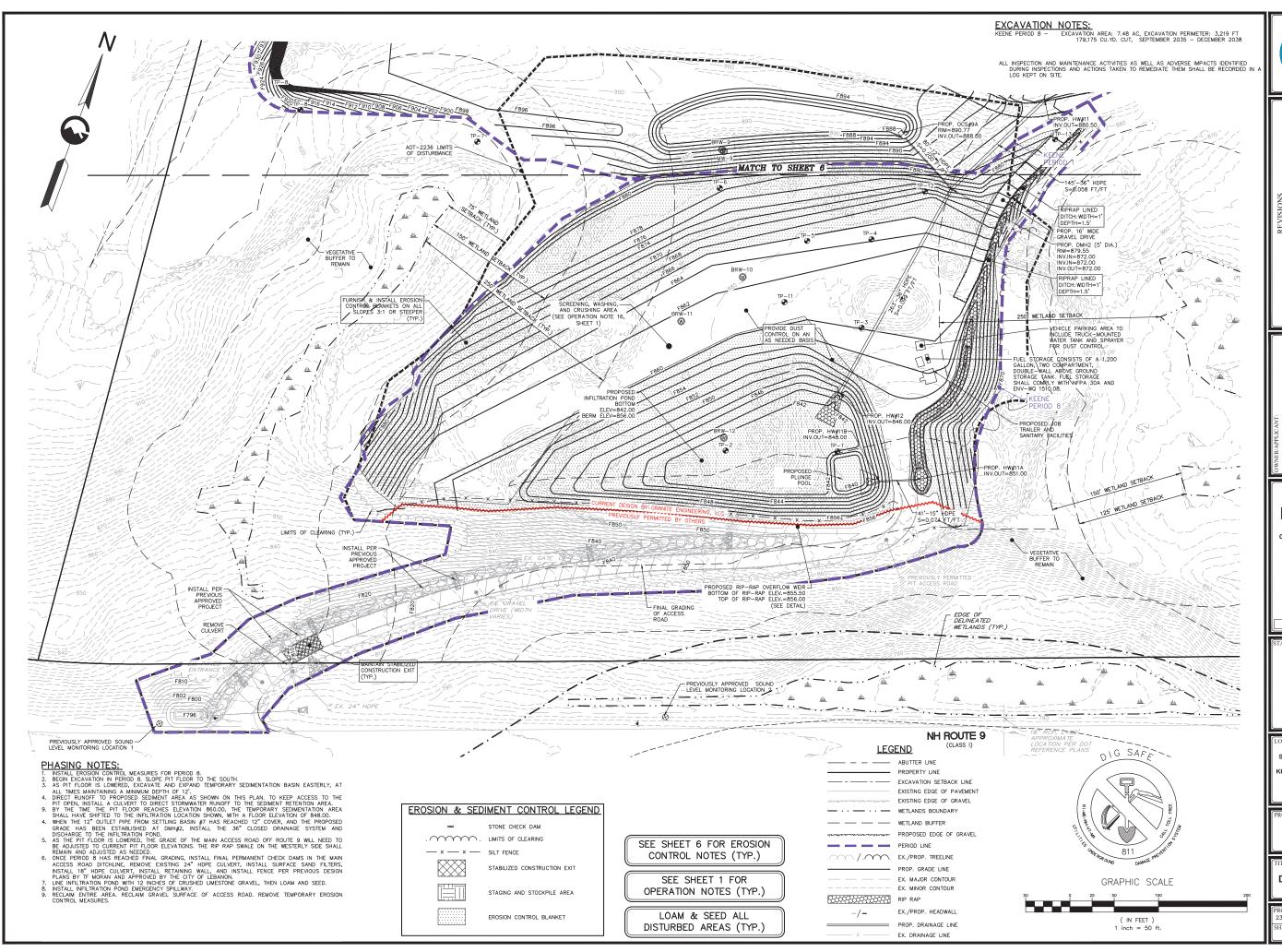
Dow Street, Tower 2, Suite 421 Manchester, New Hampshire 03101

603.518.8030

KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

GORDON SERVICES

KEENE



GRANITE

OWNER/APPLICANT: G2 HOLDINGS, LLC 250 NORTH STREET JAFFREY, NH 03452

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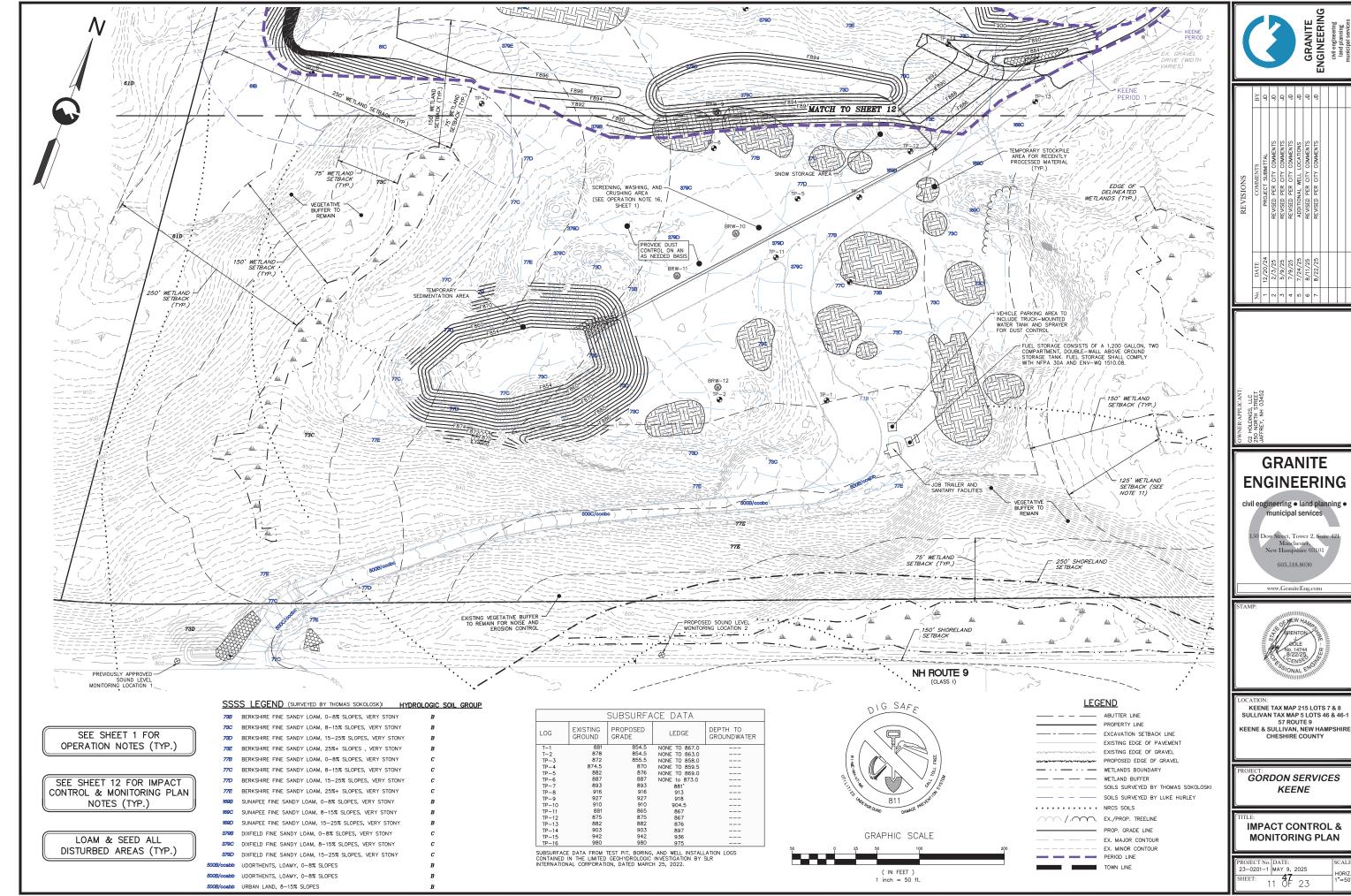


LOCATION:
KEENE TAX MAP 215 LOTS 7 & 8
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57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

OJECT:
GORDON SERVICES
KEENE

DRAINAGE & EROSION
CONTROL PLAN

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 SCALE: HORIZ. SHEET: 10 OF 23 1"=50"



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COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
DATE	12/20/24	2/3/25	5/9/25	7/9/25	7/24/25	8/11/25	8/22/25		
No.	-	2	3	4	2	9	7		

MATCH TO SHEET 11, 16

- NOISE IMPACT CONTROL AND MONITORING NOTES:

 1. NOISE LEVELS GENERATED FROM EXCAVATION ACTIVITIES SHALL NOT EXCEED THE BACKGROUND AMBIENT "A" WEIGHTED SOUND PRESSURE LEVEL EXCEEDED 90% OF THE TIME DURING THE SOUND LEVEL SAMPLING PERIOD, (HEREINAFTER" D8(A), (190")) BY MORE THAN 10 DB(A) AND IN ANY EVENT SHALL NOT EXCEED 55 DB(A) HEREINAFTER
- MORE THAN 10 DB(A) AND IN ANY EVENT SHALL NOT EXCEED 55 DB(A) HEREINAFTER (LMAXY).

 MONITORING DEVICES. ALL SOUND LEVEL MONITORING DEVICES SHALL MEET AMERICAN NATIONAL STANDARDS INSTITUTE S 1.4 TYPE 1 OR 2 STANDARDS, WITH THE DEVICE SET TO "FAST" RESPONSE. MONITORING DEVICES SHALL BE PROPERLY CALIBRATED AND MAINTAINED IN GOOD WORKING ORDER. MONITORING DEVICES SHALL INCLUDE DATA RECORDING CAPABILITIES THAT ENABLE CONTINUOUS DOCUMENTATION OF SOUND LEVELS URING THE OPERATING SOUND LEVELS SHALL BE MONITORED FROM AT LEAST 2 LOCATIONS AS DETERMINED BY THE COMMUNITY DEVELOPMENT DIRECTOR, OR THEIR DESIGNEE, WITH THE ADVICE OF OTHER CITY STAFF AND THE PLANNING BOARD'S CONSULTANT WORDMAN CACITOR HIS SECTION AS ROUND THE PLANNING BOARD'S CONSULTANT WORDMAN CACITOR HIS SECTION AS ROUND THE PROPERTY.
- NSULTANT.

 IF A MONITORING LOCATION IS SELECTED AT A POINT BEYOND THE PROPERTY
 BOUNDARY, WRITTEN PERMISSION TO USE THAT LOCATION FOR MONITORING SHALL
 BE OBTAINED FROM THE PROPERTY OWNER OF THE MONITORING SITE.
- AS NOISE-GENERATING EQUIPMENT IS RELOCATED WITHIN THE APPROVED EXCAVATION PERIMETER, NEW MONITORING LOCATIONS MAY BE SELECTED TO HELP ENSURE CONTINUED COMPELANCE WITH THE NOISE STANDARD.
- ENSURE CONTINUED COMPLIANCE WITH THE NOISE STANDARD.

 C. THE EXCAVATION OPERATOR SHALL MAINTAIN A LOG OF ALL MONITORING ACTIVITIES INDICATING THE DATE. TIME PERIOD AND LOCATION OF THE RECORDED MEASUREMENTS. HE OPERATOR BEING REPORMED ON THE SITE AT THE TIME OF THE OPERATOR O
- AMBIENT SOUND LEVELS: THE BACKGROUND AMBIENT SOUND LEVELS SHALL BE MEASURED PRIOR TO THE COMMENCEMENT OF THE INITIAL OPERATION.

 A. THE BACKGROUND SOUND LEVELS: SHALL BE MEASURED ON THE DB(A) SCALE, BY RECORDING CONTINUOUS MEASUREMENTS DURING PROPOSED OPERATING HOURS OVER 5 CONSECUTIVE BUSINESS DAYS PRIOR TO THE COMMENCEMENT OF SITE PREPARATION ACTIVITIES, AND CALCULATING THE DB(A) L(90) FOR THE ENTIRE MONITORING PERIOD. SUCH MEASUREMENTS SHALL BE PERFORMED BY A CONSULTANT HIRED BY THE PLANNING BOARD AT THE APPLICANT'S EXPENSE.

 AND CONSULTANT HIRED BY THE PLANNING BOARD AT THE APPLICANT'S EXPENSE.

 1. SPILL CONTROL PRACTICES ARE OUTLINED IN THE STORMWATER POLLUTION PREVE
- B. THE APPLICANT/OPERATOR MAY REQUEST THAT THE BACKGROUND SOUND LEVEL B.
 THE APPLICANT/OPERATOR MAY REQUEST THAT THE BACKGROUND SOUND LEVEL B.
 THE COMMUNITY DEVELOPMENT SHALL BE DONE AT A TIME SELECTED BY AND A CONSULTANT DIRECTOR IN CONSULTANT HEED BY THE PLANNING BOARD TO PERFORM THE
 MEASUREMENT AT THE APPLICANT'S EXPENSE.

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 MEASUREMENT AT T
- ONGOING MONITORING: THE APPLICANT SHALL MONITOR AT THE SELECTED MONITORING LOCATIONS THE SOUND LEVELS GENERATED BY THE OPERATION, AS FOLLOWS.

 3. A. ON AN ANNUAL BASIS, AT A TIME SELECTED BY THE COMMUNITY DEVELOPMENT DIRECTOR,

LOAM & SEED ALL

DISTURBED AREAS (TYP.)

- IN CONSULTATION WITH THE APPLICANT, SOUND LEVELS SHALL BE MONITORED AND RECORDED CONTINUOUSLY DURING OPERATING HOURS FOR A PERIOD OF NOT LESS THAN 20 CONSECUTIVE OPERATING DAYS. MONITORING SHALL BE MADE USING THE DB(A) SCLA BOT THE DB(A) SCLA BOT THE DB(A) LOLD STATE OF A SCHALL BE CALCULATED AND THE L(MAX) SOUND LEVEL THROUGHOUT EACH DAY SHALL BE CALCULATED AND ENTERED INTO A NOISE MONITORING LOG MAINTAINED BY THE APPLICANT.
- MONITORING LOG MANTAINED BY THE APPLICANT.

 B. AT ANY TIME WHEN NEW OR ADDITIONAL NOISE GENERATING COUPMENT IS PLACED INTO OPERATION FOLLOWING THE INITIAL 20-DAY MONITORING PERIOD, OR WHEN NOISE GOVERNMENT IS SELECTED TO THE PROPERTY OF THE WAY OF THE PROPERTY OF THE WAY OF
- ACTIVITIES.

 D. IN THE EVENT THAT THE MEASUREMENTS EXCEED THE NOISE STANDARDS IN THIS ARTICLE, THE APPLICANT SHALL BRING THE OPERATION INTO COMPLIANCE BY REDUCING THE NUMBER OF SOUND SOURCES CONTRIBUTING TO THE SOUND LEVEL, BY REDUCING THE NUMBER OF SOUND SOURCES CONTRIBUTING TO THE SOURCE. BY ADDING NOISE ATTENNATING STRUCTURES AROUND OR ATTACHMENTS TO THE EQUIPMENT, OR BY TAKING WHATEVER OTHER ACTIONS MAY BE NECESSARY TO BRING THE OPERATION INTO COMPLIANCE.
- a)ANY CORRECTIVE ACTION TAKEN SHALL BE CLEARLY DESCRIBED IN THE NOISE MONITORING LOG ALONG WITH A RECORD OF THE NOISE LEVEL MEASUREMENTS BEFORE AND AFTER SAID CORRECTION.
- b)ADDITIONAL NOISE LEVELS SHALL BE MONITORED FOR NO LESS THAN 5 CONSECUTIVE DAYS AFTER THE CORRECTIVE ACTION IS TAKEN.
- COMPLAINTS REGARDING THE LEVEL OF NOISE GENERATED FROM EXCAVATION OPERATIONS SHALL BE RESOLVED PER THE PROCEDURES OUTLINED IN 24.3.15.E OF THE CITY OF KEENE LAND DEVELOPMENT CODE.

- SPILL CONTROL PRACTICES ARE OUTLINED IN THE STORMWATER POLLUTION PREVENTION PREVENTION PREVENTION PREVENTION PREVENTION PREVENTION PROMISES BY STANDARD PROCESS, PRIMARILY CONSISTING OF PETROLEUM-BASED OILS, LUBRICANTS, AND GASQLINE-BASED FUELS. THESE SUBSTANCES MUST BE STORED SECURELY IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND MUST BE ACCOMPANIED BY MATERIAL, SAFETY DATA SHEETS AND SPILL RESPONSE MATERIALS, STRICT PRECAUTIONS MUST BE TAKEN DURING ON-SITE FUELING OPERATIONS TO PREVENT SPILLS AND OVERFILLING.

SEE SHEET 1 FOR SEE SHEET 4 FOR FROSION OPERATION NOTES (TYP.) CONTROL NOTES (TYP.)

- DUST CONTROL & MONITORING NOTES:

 1. THE SITE SHALL OPERATE IN A MANNER THAT PREVENTS FUGITIVE DUST EMISSIONS PUBLISHED TO NEW HAMPSHIRE CODE OF ADMINISTRATIVE RULES ENV-A 1002, FUGITIVE DUST CONTROL PRACTICES ARE OUTLINED IN THE STORMWATER POLLUTION PREVENTION
- . DUST CONTROL PRACTICES ARE OUTLINED IN THE STOKMWATER POLLUTION PREVENTION PLANS (SWPPP).

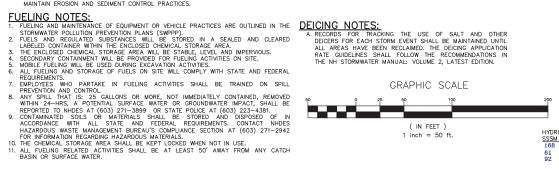
 DUST CONTROL ACTIVITIES AND DEVICES SHALL BE INCORPORATED INTO THE EXCAVATION OPERATION, ON THE SITE AND ON THE ACCESS DRIVEWAY, IN A MANNER THAT MINIMIZES GENERATION OF AIRBORNE DUST OR TRANSPORTATION OF DUST OR MUD OFF THE SITE ONTO THE ADJACENT ROADWAYS.

 A. VISUAL MONITORING OF AIRBORNE DUST SHALL BE DONE ON AN ONGOING BASIS.
- A VISUAL MOVINGHING OF AIRBORNE DOST SHALL BE DONE ON AN OVINGHING BASIS.

 B. DUST CONTROL MEASURES SUCH AS APPLYING WATER TO ACCESS DRIVEWAYS AND OTHER AREAS WITHIN THE EXCAVATION PERIMETER, WASHING DIRT FROM TRUCK TIRES, OR OTHER MEASURES AS MAY BE DEEMED NECESSARY, SHALL BE EMPLOYED TO MINIMIZE THE GENERATION OF AIRBORNE DUST, AND/OR THE TRANSPORTATION OF DIRT/MUD OFF THE SITE ONTO ADJACENT ROADWAYS.
- C. DUST CONTROL WILL BE ACCOMPLSHED USING A TRUCK-MOUNTED WATER TANK AND SPRAY SYSTEM AS NEEDED.
- INSPECTION OF ACCESS DRIVEWAY STABILIZED CONSTRUCTION ENTRANCES AND OTHER EROSION CONTROL MEASURES, DESIGNED TO ELIMINATE THE DEPOSIT OF DUST OR MUDD ONTO PUBLIC STREETS, SHALL BE CONDUCTED ON A WEEKLY BASIS TO ENSURE PROPER FUNCTIONING. MAINTENANCE OF THESE ENTRANCES SHALL BE PERFORMED AS NECESSARY AND ANY DIRT OR MUD DEPOSITED ON PUBLIC STREETS SHALL BE
- INSPECTION AND MAINTAINE OF DUST AND DIET CONTROL STRUCTURES AND DEVICES, AND CLEAN UP OF DIET DEPOSITED ON ROADWAYS LEADING FROM THE STITLE THE OPERATION AND MAINTENANCE MANUAL, LOCATED WITHIN THE STORMWATER MAINGAGEMENT REPORT, SHALL BE USED FOR INSTURCTIONS OF HOW TO INSPECT AND MAINTAIN EROSION AND SEDIMENT CONTROL PRACTICES.

		SUBSURFA	CE DATA	
LOG	EXISTING GROUND	PROPOSED GRADE	LEDGE	DEPTH TO GROUNDWATER
T-1	881	854.5	NONE TO 867.0	
T-2	878	854.5	NONE TO 863.0	
TP-3	872	855.5	NONE TO 858.0	
TP-4	874.5	870	NONE TO 859.5	
TP-5	882	876	NONE TO 869.0	
TP-6	887	887	NONE to 873.0	
TP-7	893	893	881'	
TP-8	916	916	913	
TP-9	927	927	918	
TP-10	910	910	904.5	
TP-11	881	865	867	
TP-12	875	875	867	
TP-13	882	882	876	
TP-14	903	903	897	
TP-15	942	942	936	
TP-16	980	980	975	

NECESSARY AND ANY DIRT OR MUD DEPOSITED ON PUBLIC STREETS SHALL BE SUBSURFACE DATA FROM TEST PIT, BORING, AND WELL INSTALLATION LOGS CONTAINED IN THE LIMITED GEOHYPOROLOGIC INVESTIGATION BY SLR INTERNATIONAL CORPORATION, DATED MARCH 25, 2022.



60

61

73

77

161

THIS MAP PRODUCT IS WITHIN THE TECHNICAL STANDARDS OF THE NATIONAL COOPERATIVE SOIL SURVEY. IT IS A SPECIAL PURPOSE PRODUCT, INTENDED FOR INFLIRATION REQUIREMENTS OF TERRAIN SUREAU, IT WAS SHOULD BE ALTERATION OF TERRAIN SUREAU, IT WAS PRODUCT OF THE USBANDATURAL RESOURCES CONSERVATION SERVICE. THERE IS A REPORT THAT ACCOMPANIES THIS MAP.

SITE SPECIFIC SOIL SURVEY NOTES:

LEGEND ABUTTER LINE PROPERTY LINE EXCAVATION SETBACK LINE EXISTING EDGE OF PAVEMENT EXISTING EDGE OF GRAVEL PROPOSED EDGE OF GRAVEL

- · · - · · - WETLANDS BOUNDARY

NRCS SOILS LEGEND

COLTON GRAVELLY SANDY LOAM, RATED A

TUNBRIDGE-BERKSHIRE COMPLEX, RATED C

BERKSHIRE FINE SANDY LOAM, RATED B

TUNBRIDGE-LYMAN-ROCK OUTCROP COMPLEX, RATED C

LYMAN-TUNBRIDGE-ROCK OUTCROP COMPLEX, RATED D SUNAPEE FINE SANDY LOAM, RATED C

MARLOW FINE SANDY LOAM, VERY STONY, RATED C

· · · · · · · · · · · NRCS SOILS

WETLAND BUFFER

- PROP. GRADE LINE - FX. MAJOR CONTOUR EX. MINOR CONTOUR TOWN LINE

SOILS SURVEYED BY THOMAS SOKOLOSK

SOILS SURVEYED BY LUKE HURLEY

THE SITE SPECIFIC SOIL SURVEY (SSSS) WAS PRODUCED JULY 15, 2024, AND WAS PREPARED BY LUKE HURLEY, CSS #095M, HURLEY ENVIRONMENTAL AND LAND PLANNING, LLC.

SOILS WERE IDENTIFIED WITH THE NEW HAMPSHIRE STATE—WIDE NUMERICAL SOILS LEGEND, USDA NRCS, DURHAM, NH. ISSUE #10, JANUARY 2011. THE NUMERIC LEGEND WAS AMENDED TO IDENTIFY THE CORRECT SOIL COMPONENTS OF THE COMPLEX.

HYDROLOGIC SOIL GROUP FROM KSAT VALUES FOR NEW HAMPSHIRE SOILS, SOCIETY OF SOIL SCIENTIST OF NEW ENGLAND, SPECIAL PUBLICATION NO. 5, SEPTEMBER, 2009.

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HYDROLOGIC		11100 0144	0011 001
SSSM SYM.	SSS MAP NAME	HISS SYM.	SOIL GRE
168	SUNAPEE	321	В
61	TURNBRIDGE LYMAN ROCK OUTCR	OP 224/227	С
92	LYMAN	224	D

SLOPE PERIOD: 0-8% B 8-15% C 15-25% D 25%+ E



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KEENE TAX MAP 215 LOTS 7 & 8 SULLIVAN TAX MAP 5 LOTS 46 & 46-1 57 ROUTE 9

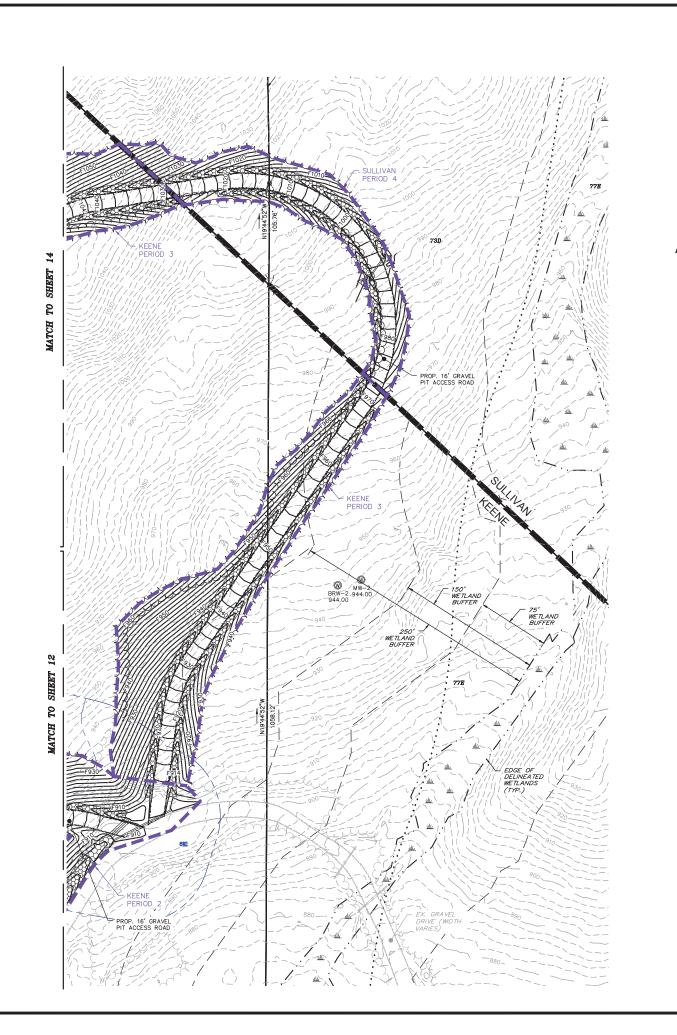
CHESHIRE COUNTY

KEENE & SULLIVAN, NEW HAMPSHIRE

GORDON SERVICES KEENE

IMPACT CONTROL & MONITORING PLAN

23-0201-1 MAY 9, 2025 12 OF 23



SEE SHEET 1 FOR OPERATION NOTES (TYP.)

SEE SHEET 12 FOR IMPACT CONTROL & MONITORING PLAN NOTES (TYP.)

LOAM & SEED ALL

DISTURBED AREAS (TYP.)



LEGEND ABUTTER LINE PROPERTY LINE EXCAVATION SETBACK LINE EXISTING EDGE OF PAVEMENT EXISTING EDGE OF GRAVEL PROPOSED EDGE OF GRAVEL - · · - · · · WETLANDS BOUNDARY --- WETLAND BUFFER SOILS SURVEYED BY THOMAS SOKOLOSKI - - - SOILS SURVEYED BY LUKE HURLEY · · · · · · · · · · · NRCS SOILS EX./PROP. TREELINE PROP. GRADE LINE EX. MAJOR CONTOUR EX. MINOR CONTOUR PERIOD LINE

NRCS SOILS LEGEND

TOWN LINE

COLTON GRAVELLY SANDY LOAM, RATED A TUNBRIDGE-BERKSHIRE COMPLEX, RATED ${\tt C}$ TUNBRIDGE-LYMAN-ROCK OUTCROP COMPLEX, RATED C BERKSHIRE FINE SANDY LOAM, RATED B MARLOW FINE SANDY LOAM, VERY STONY, RATED C LYMAN-TUNBRIDGE-ROCK OUTCROP COMPLEX, RATED D 161 SUNAPEE FINE SANDY LOAM, RATED C

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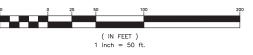
HYDROLOGIC SSSM SYM.	SSS MAP NAME	HISS SYM.	SOIL GRP.
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61	TURNBRIDGE LYMAN ROCK OUTCROP	224/227	С
92	LYMAN	224	D

SLOPE PERIOD: 0-8% B 8-15% C 15-25% D 25%+ E





GRAPHIC SCALE





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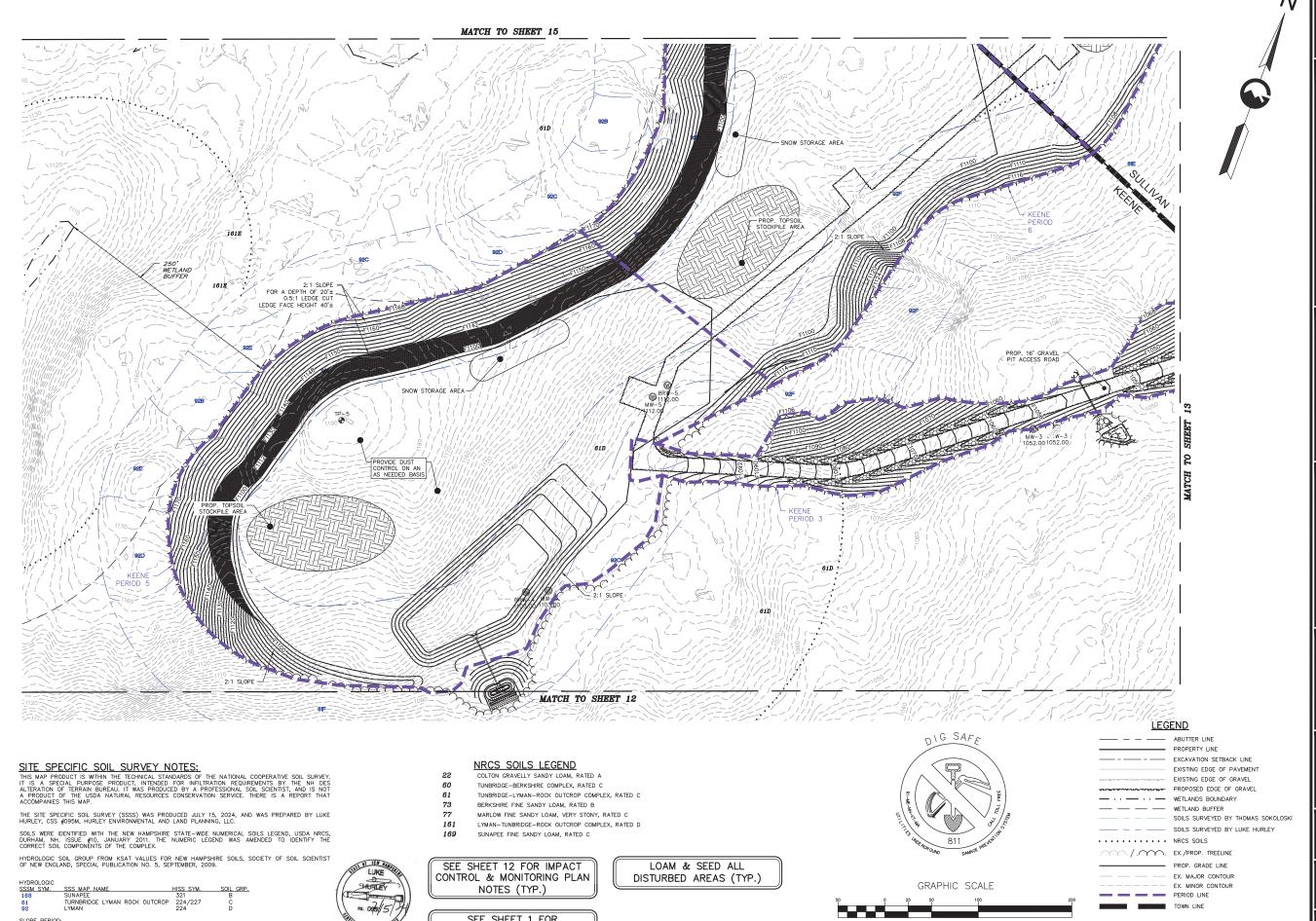


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CHESHIRE COUNTY

GORDON SERVICES KEENE

IMPACT CONTROL & **MONITORING PLAN**

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 13 OF 23



SEE SHEET 1 FOR

OPERATION NOTES (TYP.)

SLOPE PERIOD: 0-8% B 8-15% C 15-25% D 25%+ E

		REVISIONS	
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CHESHIRE COUNTY

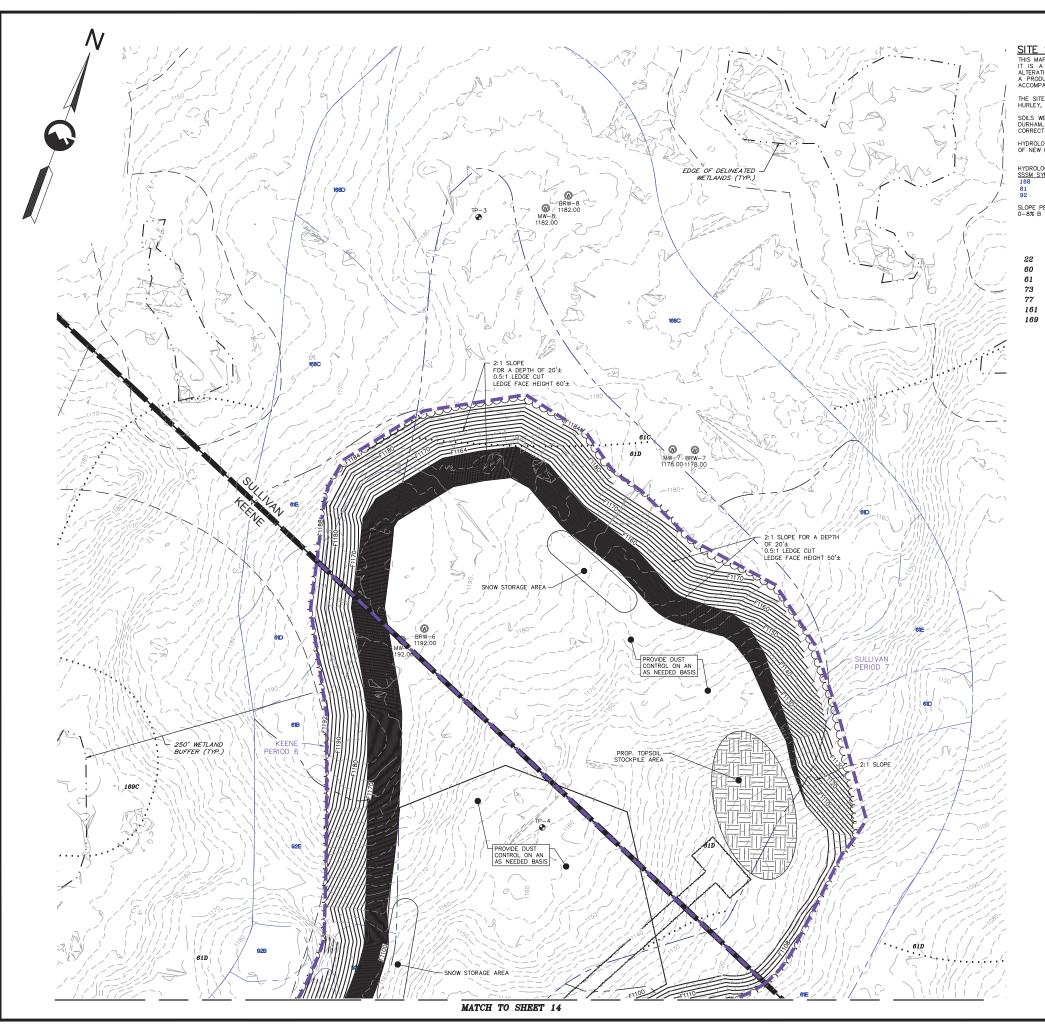
GORDON SERVICES KEENE

IMPACT CONTROL & **MONITORING PLAN**

PERIOD LINE

(IN FEET)

23-0201-1 MAY 9, 2025 14 OF 23



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NRCS SOILS LEGEND
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LYMAN-TUNBRIDGE-ROCK OUTCROP COMPLEX, RATED D

SUNAPEE FINE SANDY LOAM, RATED C



SEE SHEET 12 FOR IMPACT CONTROL & MONITORING PLAN NOTES (TYP.)

SEE SHEET 1 FOR OPERATION NOTES (TYP.)

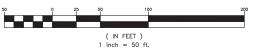
LOAM & SEED ALL DISTURBED AREAS (TYP.)

LEGEND

	ABUTTER LINE
	PROPERTY LINE
	EXCAVATION SETBACK LINE
	EXISTING EDGE OF PAVEMENT
20128/Juleun terretur	EXISTING EDGE OF GRAVEL
SAMMANNA MASAMANA	PROPOSED EDGE OF GRAVEL
	WETLANDS BOUNDARY
	WETLAND BUFFER
	SOILS SURVEYED BY THOMAS SOKOLOS
	SOILS SURVEYED BY LUKE HURLEY
	NRCS SOILS
m./.m.	EX./PROP. TREELINE
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PROP. GRADE LINE EX. MAJOR CONTOUR EX. MINOR CONTOUR PERIOD LINE

GRAPHIC SCALE





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DATE	12/20/24	2/3/25	5/9/25	7/9/25	7/24/25	8/11/25	8/22/25		
No.	-	2	3	4	2	9	7		

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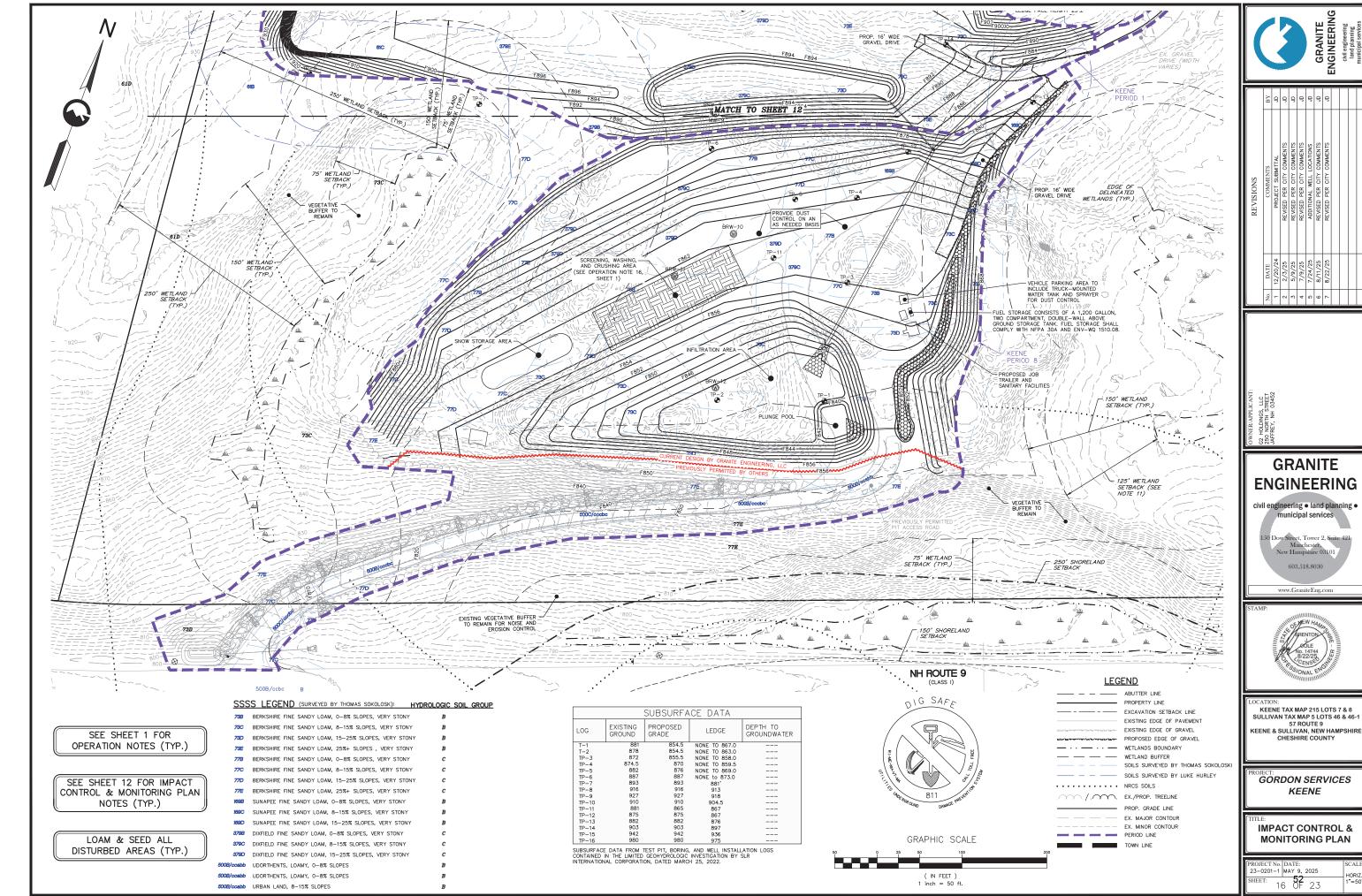


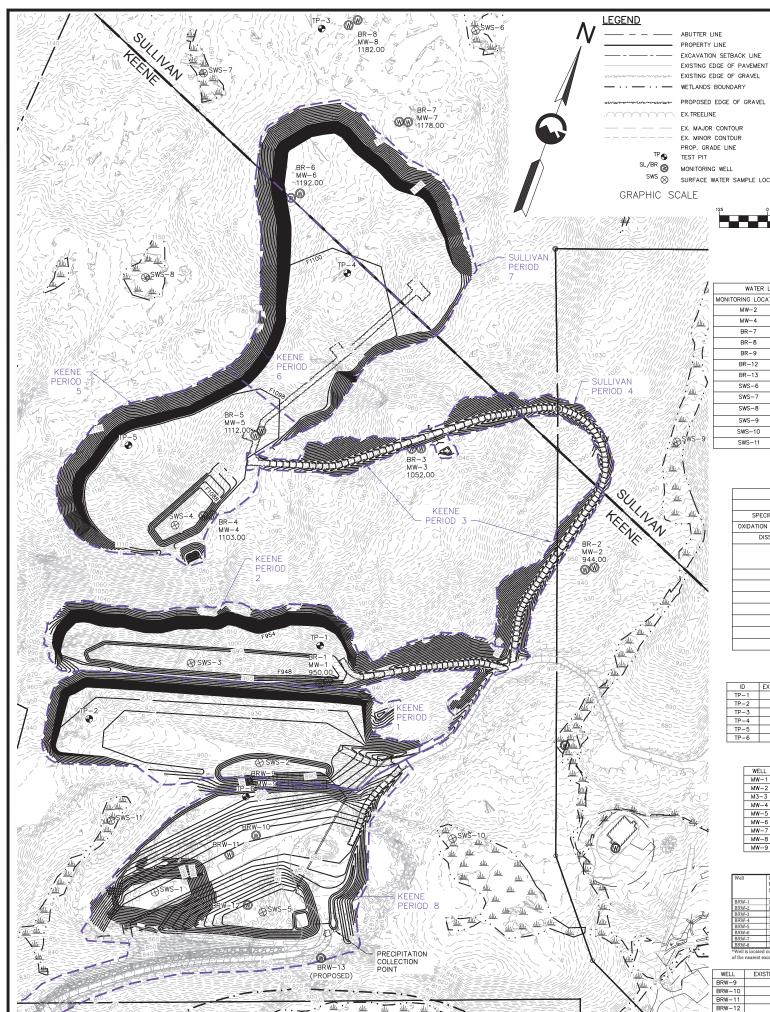
LOCATION:
KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

GORDON SERVICES KEENE

IMPACT CONTROL & **MONITORING PLAN**

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 15 OF 23





OIG SAFE

(IN FEET) 1 inch = 125 ft

EX. MAJOR CONTOUR EX. MINOR CONTOUR PROP. GRADE LINE SWS SURFACE WATER SAMPLE LOCATION

> WATER LEVEL MONITORING TORING LOCATION FREQUENCY MONTHLY MONTHLY BR-8 MONTHLY MONTHLY BR-9 BR-12 MONTHLY BR-13 MONTHLY SWS-6 MONTHLY SWS-7 MONTHLY MONTHLY SWS-8 MONTHLY SWS-9

> > SWS-11

AMD FORMATION S	TANDARDS
<u>ANALYTE</u>	STANDARD
SPECIFIC CONDUCTANCE	NO STANDARD
OXIDATION REDUCTION POTENTIAL	NO STANDARD
DISSOLVED OXYGEN	75%/5 mg/L
TURBIDITY	10 NTU (BEYOND NATURALLY OCCURRING CONDITIONS)
pН	6.0 SU
ARSENIC	5 μg/L
COPPER	1,300 µg/L
IRON	300 µg/L
MANGANESE	300 μg/L
NICKEL	100 µg/L
LEAD	15 μg/L

MONTHI Y

MONTHI Y

TEST PITS

ID	EXISTING GRADE	LEDGE	GROUNDWATER	E.S.H.W.T.
TP-1	986.0	3.1'	NONE FOUND	NONE FOUND
TP-2	926.0	NONE	NONE FOUND	20"
TP-3	1180.0	NONE	NONE FOUND	NONE FOUND
TP-4	1158.0	NONE	NONE FOUND	32"
TP-5	1190.0	0.5'	NONE FOUND	NONE FOUND
TP-6	887.0	873.0	NONE FOUND	NONE FOUND

OVERBURDEN WELLS

WELL	EXISTING GRADE	LEDGE	GROUNDWATER
MW-1	950.0	3.3'	NONE FOUND
MW-2	944.0	12'	NONE FOUND
M3-3	1052.0	14.2'	NONE FOUND
MW-4	1103.0	3.1'	NONE FOUND
MW-5	1112.0	5'	NONE FOUND
MW-6	1192.0	0.9'	NONE FOUND
MW-7	1178.0	1.9'	NONE FOUND
MW-8	1182.0	1.1'	NONE FOUND
MW-9	884.38	XX	XX

BEDROCK WELLS

Well	Ground	Bedrock	Depth/Bottom	Proposed Pit	Groundwater
	Elevation	Depth	Elevation	Floor	Elevation
	(ft AMSL)	(feet)	(feet/ ft	Elevation	(ft AMSL)
			AMSL)	(ft AMSL)	
BRW-1	950	3	54/896	950	DRY
BRW-2	944	12	62/882	940	DRY
BRW-3	1052	14	51/1,001	1,050	DRY
BRW-4	1,103	3	81/1,022	1,098	DRY
BRW-5	1,112	3	141/971	1,098	DRY
BRW-6	1,192	1	142/1,050	1,098	DRY
BRW-7	1,178	1.9	141/1,037	1,098*	1,177.04
BRW-8	1,182	1	141/1,041	1,098*	1,179.16
Well is located	outside of project	area. The pit flo	or elevation that	is noted is the pr	oposed elevation
of the nearest ex	cavation.				

884.31 50' 877.91 85' 854.44 879.94 884.34 881 39 885 19 839.96

ON-SITE WATER QUALITY MONITORING NOTES (ARD MONITORING)

PH, SPECIFIC CONDUCTANCE, OXIDATION, REDECTION POTENTIAL, DISSOLVED OXYGEN AND TURBITY AND LABRATORY ANALYSIS OF DISSOLVED AND TOTAL METALS INCLUDING; ARSENIC, COPPER, IRON, MAGNESE, NICKEL AND LEAD WILL BE TESTED AT EACH LOCATION

INITIAL RESPONSE ACTION — AMD DETECTION

IF A SURFACE AND/OR GROUNDWATER SAMPLING LOCATION PRESENTS RESULTS THAT ARE INDICATIVE OF THE FORMATION OF ACID MINE DRAINAGE, NHDES AND THE CITY OF KEENE WILL BE NOTHED IMMEDIATELY. FURTHERMORE, THE FOLLOWING IMMEDIATE INTITIAL RESPONSE ACTIONS WILL BE IMPLEMENTED.

1. ALL ACTIVE QUARRYING/MINING OPERATIONS OCCURRING IN THE AFFECTED AREA WILL CEASE AND EXPOSED BEDROCK SURFACES SHALL BE EXPEDITIOSLY RESTORED TO HAVE A MINIMUM THICKNESS OF 3-FEET OF COVER MATERIAL. THE COVER MATERIAL SHALL CONSTS OF A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE SURFED FOR A MINIMUM OF 30% CLAY CONTENT. COVER MATERIAL SHALL BE ARRORED WITH 1-FOOT OF 2-INCH MINIUS, CRUSHED, LIMESTONE GRAVEL.

2. ANY DOWNSLOPE AFFECTED PRAINAGES WHICH MAY BE CONTRIBUTION COVERYING ACID MINE DRAINAGE SHALL BE ARRORED WITH 1-FOOT OF 2-INCH MINIUS, CRUSHED, LIMESTONE GRAVEL.

3. ALL SURFACE WATER WITHIN Y—MILE DOWNGRADIENT OF THE DETECTED ACID MINE DRAINAGE SHALL BE SAMPLED WITHIN 2-WEEKS OF THE INITIAL DETECTION AND BE INCLUDED IN THE SURFACE WATER MONITORING PROGRAM.

4. SAMPLING OF ALL DOMESTIC WATER SUPPLY WELLS WITHIN Y—MILE OF THE AFFECTED BARE FOR ACID MINE DRAINAGE PARAMETERS WILL OCCUR WITHIN 2-WEEKS OF THE INITIAL DETECTION AND CONTINUE TO BE SAMPLED ON A QUARTERY USE AND CONTINUE TO BE SAMPLED ON A QUARTERY USE AND CONTINUE TO BE SAMPLED ON A QUARTERY USE AND CONTINUE OF A MINIMUM OF FOREIGN OF A MINIMUM OF A MIN

EE INITIATED.

A. THE GROUNDWATER QUALITY ASSESSMENT SHALL INCLUDE THE INSTALLATION OF A MINIMUM OF THREE (3) MONITORING WELLS; ONE UPGRADIENT OF THE AFFECTED SURFACE WATER, AND TWO DOWN-GRADIENT OF THE AFFECTED SURFACE WATER, ADDITIONAL MONITORING WELLS MAY BE REQUIRED TO DETERMINE THE HORIZONTAL, AND VERTICAL DISTRIBUTION OF THE GROUNDWATER MAPACTS AS BELIEVED AND VERTICAL DISTRIBUTION OF THE GROUNDWATER MAPACTS AND THE ADDITIONAL OF THE ACID MINE DRAINAGE FARMETERS LISTED ABOVE. A SECOND, CONFIRMATORY ROUND OF SAMPLING WILL OCCUR 2-WEEKS AFTER THE INITIAL SAMPLING ROUND, MONITORING WELLS WILL BE SAMPLED ON A QUARTERLY BASIS IF ACID MINE DRAINAGE IMPACTS ARE DETECTED.

OFF-SITE WATER QUALITY MONITORING NOTES

NOTIFICATIONS WILL BE MADE TO LANDOWNERS WITH THE OPTION TO ALLOW OR DECLINE N BASELINE MONITORING WILL OCCUR A MINIMUM OF 14 CALENDAR DAYS APART. SAMPLES WILL BE ANALYZED FOR VOLATILE ORGANIC COMPOUNDS AND NITRATE. RESULTS WILL BE SENT TO THE LAND OWNER, THE CITY OF KEENE, & TOWN OF SULLIVAN.

ON-COING MONITORING

6. BI-ANNUAL BASIS OF WELLS DURING THE TERM OF THE PERMIT.

7. BI-ANNUAL BASIS OF WELLS TWO (2) YEARS FOLLOWING THE CEASE OF OPERATIONS AT THE SITE AND

7. BI-ÁNNIÁL BASÍS ÓF WELLS TWO (2) YEARS FOLLOWING THE CEASE OF OPERATIONS AT THE SITE AND RECLAMATION.
4. SAMPLES WILL BE ANALYZED FOR VOLATILE ORGANIC COMPOUNDS AND NITRATE.
5. RESULTS WILL BE SENT TO THE LAND OWNER, THE CITY OF KEENE, & TOWN OF SULLIVAN.
6. ADVERSE IMPACTS.
6. DRINKING WATER RESULTS WILL BE COMPARED TO THE NHOES AMBIENT GROUNDWATER QUALITY STANDARDS (AGGS).
7. IF ADVERSE IMPACTS APE NOTED. THE APPLICANT WILL IMMEDIATELY DE NOTIFIED.
8. NHOES, THE CITY OF KEENE, AND TOWN OF SULLIVAN WILL BE NOTIFIED.
9. IF MONITORING INDICATES THE EXCAVATION ACTIVITIES CAUSED THE IDENTIFIED CONTAMINATION, A LICENSED NH WELL CONTRACTOR WILL BE MIMEDIATELY PETAMED FOR INSTALLATION OF A NEW WATER SUPPLY WELL IN AN AREA THAT HAS NOT BEEN IMPACTED BY CONTAMINATION.
10. COST OF WELL WILL BE THE SOLE RESPONSIBILITY OF THE APPLICANT.

BEST MANAGEMENT PRACTICES FOR BLASTING

ALL ACTIVITIES RELATED TO BLASTING SHALL FOLLOW BEST MANAGEMENT PRACTICES (BMPS) TO PREVENT CONTAMINATION OF GROUNDWATER INCLUDING PREPARING, REVIEWING AND FOLLOWING AN APPROVED BLASTING PLAN; PROPER DRILLING, EXPLOSIVE HANDING AND LOADING PROCEDURES; OBSERVING THE ENTIRE BLASTING PROCEDURES; EVALUATING BLASTING PERFORMANCE; AND HANDLING AND STORAGE OF BLASTED ROCK.

(1) LOADING PRACTICES. THE FOLLOWING BLASTHOLE LOADING PRACTICES TO MINIMIZE ENVIRONMENTAL EFFECTS SHALL BE

HANDLING AND STORAGE OF BLASTED ROCK.

(1) LOADING PRACTICES. THE FOLLOWING BLASTHOLE LOADING PRACTICES TO MINIMIZE ENVIRONMENTAL EFFECTS SHALL BE FOLLOWED:

(a) DRILLI INDICATE DEPTHS AND LENGTHS OF VOIDS. CANTES, AND FAULT ZONES OR OTHER WEAK ZONES ENCOUNTERED AS WELL AS GROUNDWATER CONDITIONS. F. A FRACTURE OR WATER BEARING ZONE IS ENCOUNTERED IN A BOREHOLE, NO BLASTING SHALL DOCUMENT AND LENGTHS OF THE PROPERTY VEHICLE, OR PLACED IN SECURE OR WATER BEARING ZONE IS ENCOUNTERED IN A BOREHOLE, REPOSITE PRODUCTS SHALL BE MANAGED ON SITE SO THAT THEY ARE EITHER USED IN THE BORCHOLE, RETURNED TO THE DELIVERY VEHICLE, OR PLACED IN SECURE CONTAINERS FOR OFF SITE DISPOSAL.

(c) SPILLAGE AROUND THE BOREHOLE SHALL EITHER BE PLACED IN THE BORCHOLE OR CLEARD UP AND RETURNED TO AN APPROPRIATE VEHICLE FOR HANDLING OF PLACEMENT IN SECURED CONTAINERS FOR OFF-SITE DISPOSAL.

(d) OVERNIGHT, UNITSES WEATHER OR OTHER SAFETY CONCERNS REASONABLY DICTATE THAT DETONATION SHOULD BE POSTFONED.

(e) LOADING EQUIPMENT SHALL BE CLEANED IN AN AREA WHERE WASTEWATER CAN BE PROPERTY CONTAINED AND HANDLED IN A MANNER THAT PREVENTS RELEASE OF CONTAINNANTS TO THE ENVIRONMENT.

(f) EXPLOSIVES SHALL BE LOADED TO MAINTAIN GOOD CONTINUITY IN THE COLUUM LOAD TO PROMOTE COMPLETE DETONATION, INDUSTRY ACCEPTED LOADING PRACTICES FOR PRIMING, STEMMING, DECKING AND COLUMN RISE NEED TO CONTAINNANTS TO THE ENVIRONMENT.

(2) EXPLOSIVE SELECTION THE FOLLOWING BUPS SHALL BE FOLLOWED TO REDUCE THE POTENTIAL FOR ROOUNDWATER CONTAINNANTS TO THE SITE CONTAINNANTON WHEN EXPLOSIVES ARE USED!

(a) EXPLOSIVE PRODUCTS SHALL BE SELECTED THAT HAVE THE APPROPRIATE FOR SITE CONDITIONS AND SAFE BLAST EXECUTION.

(b) EXPLOSIVE PRODUCTS SHALL BE SELECTED THAT HAVE THE APPROPRIATE FOR THE PROTUNTIAL FOR ROOUNDWATER.

(c) READON THE MINIMAL SHALL BE SELECTED THAT HAVE THE APPROPRIATE FOR THE FORM THIS SITE CONTAINNANTS TO THE ENVIRONMENT. THE SITE CONTAINNANTS TO THE ENVIRONMENT. THE SITE CONTAINNANTS TO THE ENVIRONMENT WAS THE SITE OF THE PROTUCT OF THE PRODUCTS SHALL BE SELE

AMD MONITORING

Location	Type	Frequency	Parameters	Applicable Standards
BRW-7	Groundwater	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Or 600 AGQS
BRW-8	Groundwater	Quarterly (Jan, Apr. Jul, Oct); pH monthly	Field + Metals suite	Env-Or 600 AGQS
BRW-9	Groundwater	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Or 600 AGQS
BRW-12	Groundwater	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Or 600 AGQS
BRW-13 (proposed)	Groundwater	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Or 600 AGQS
Period 8 infiltration basin	Surface water	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Wq 1700
Additional infiltration basins (as constructed)	Surface water	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Wq 1700
Downgradient discharge station	Surface water	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Wq 1700
SWS-6	Surface water	Quarterly (Jan, Apr, Jul, Oct); pH monthly	Field + Metals suite	Env-Wq 1700

- SC = Specific Conductance; ORP = Oxidation-Reduction Potential; DO = Dissolved Oxygen

Baseline monitoring will consist of two (2) rounds. £14 days agar, prior to excavation.

Transition to biannual monitoring (April, October) may occur after four consecutive quarters demonstrate no evidence of AMD.

—Phand DO are evaluated as field inclicators, not numeric standards, except where narrative criteria apply in surface water.



	В	9	9	9	9	딩	9	5		
SNOISI AN	COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
	DATE	12/20/24	2/3/25	5/9/55	7/9/25	7/24/25	8/11/25	8/22/25		
	No.	-	2	3	4	2	9	7		

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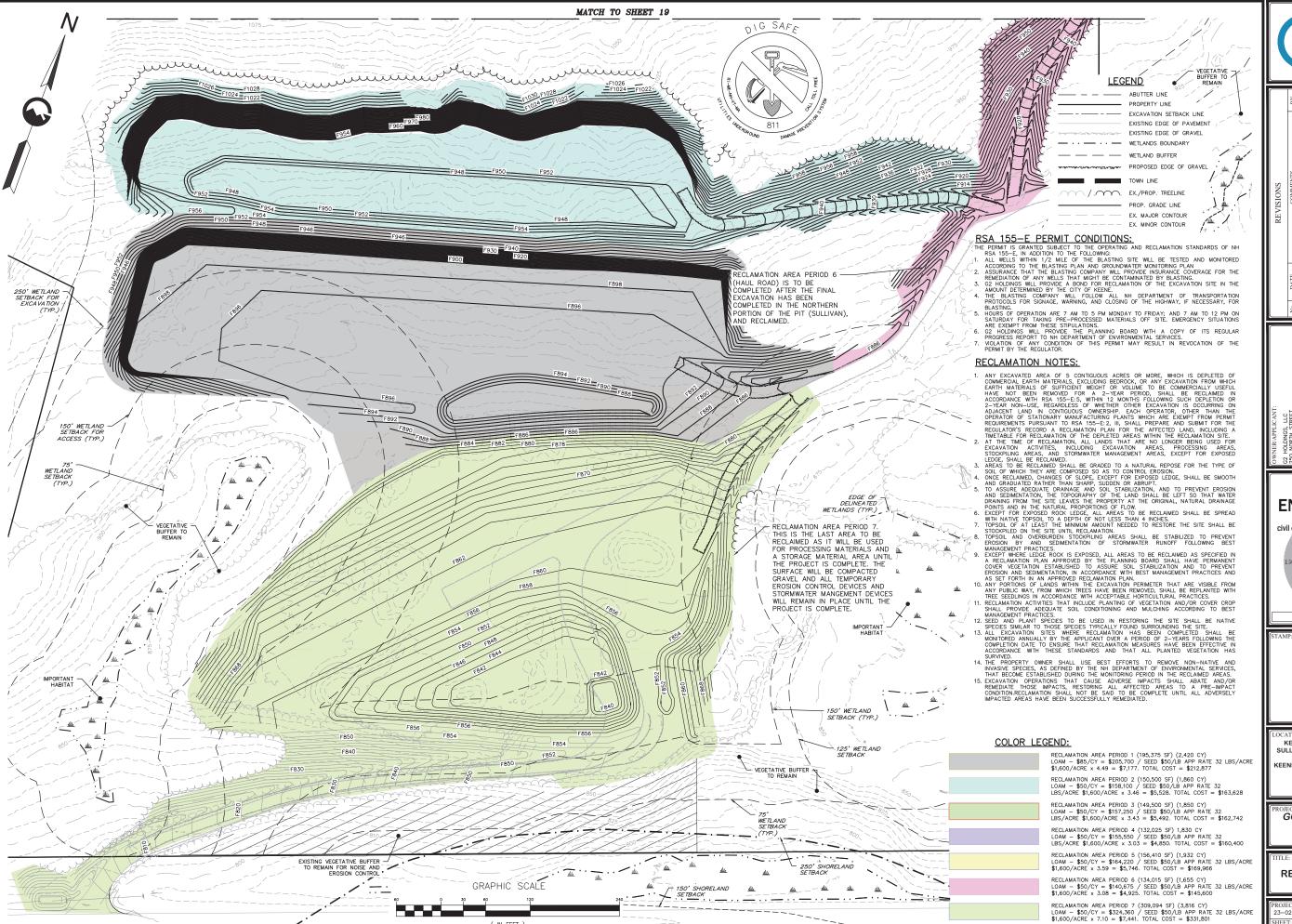


KEENE TAX MAP 215 LOTS 7 & 8 SULLIVAN TAX MAP 5 LOTS 46 & 46-1 57 ROUTE 9 KEENE & SULLIVAN, NEW HAMPSHIRE CHESHIRE COUNTY

GORDON SERVICES KEENE

MONITORING PLAN

PROJECT	No. DATE:	SCALE:
23-020	1-1 MAY 9, 2025	HORIZ.
SHEET:	17 53	1"=125"



(IN FEET) 1 inch = 60 f



	ВУ	9	9	号	9	9	9	9		
REVISIONS	COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
	DATE	12/20/24	2/3/25	5/9/25	7/9/25	7/24/25	8/11/25	8/22/25		
	No.	-	2	ы	4	2	9	7		

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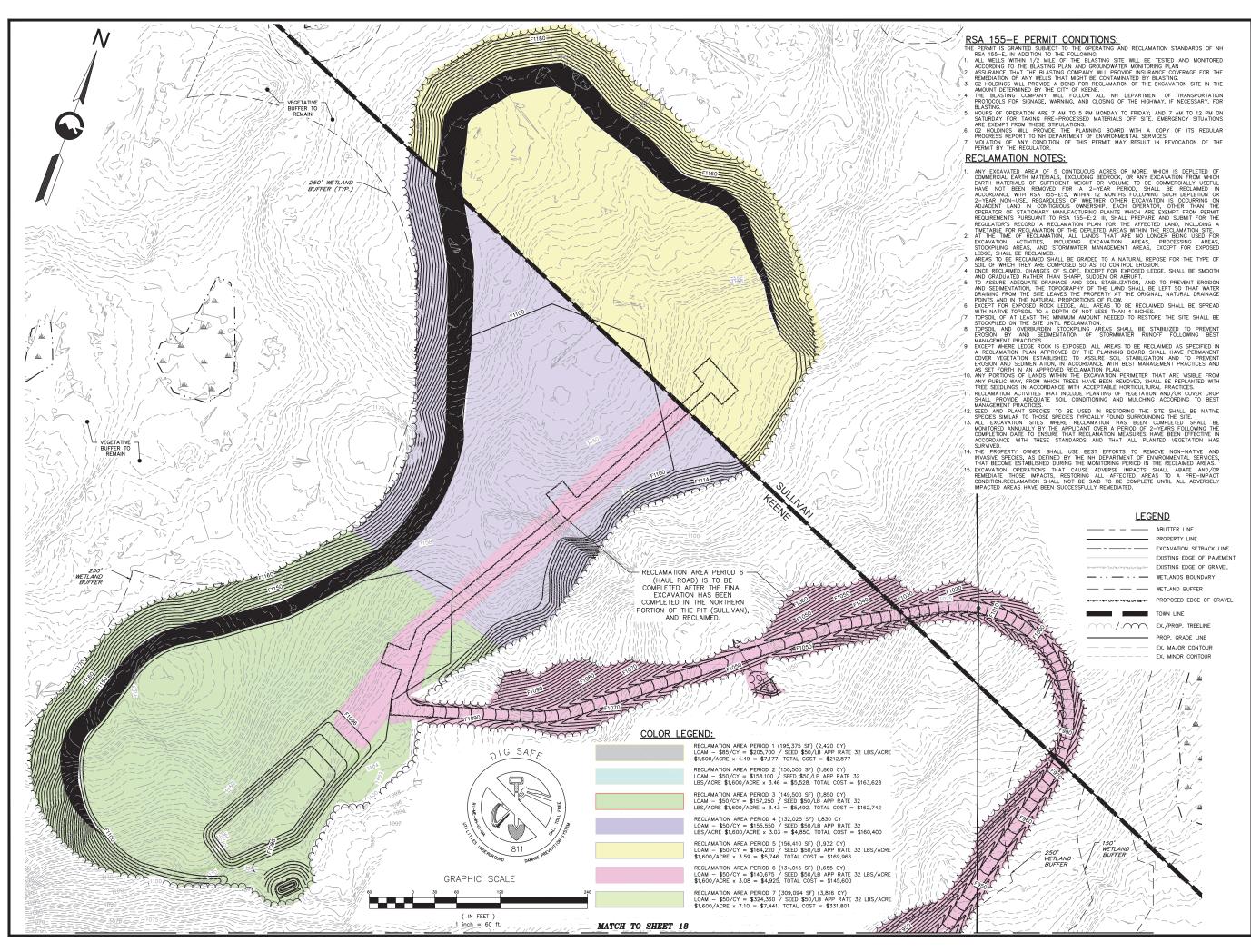
KEENE TAX MAP 215 LOTS 7 & 8 SULLIVAN TAX MAP 5 LOTS 46 & 46-1 57 ROUTE 9 KEENE & SULLIVAN, NEW HAMPSHIRE

CHESHIRE COUNTY

GORDON SERVICES KEENE

RECLAMATION PLAN

OJECT No. DATE:	SCALE:
3-0201-1 MAY 9, 2025	HORIZ
EET: 40 54 07	HORIZ. 1"=60"
18 OF 23	J





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ВУ	9	유	9	9	9	9	9		
COMMENTS	PROJECT SUBMITTAL	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS	ADDITIONAL WELL LOCATIONS	REVISED PER CITY COMMENTS	REVISED PER CITY COMMENTS		
DATE	12/20/24	2/3/25	5/9/25	7/9/25	7/24/25	8/11/25	8/22/25		
Š.	-	2	23	4	2	9	7		

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OCATION:
KEENE TAX MAP 215 LOTS 7 & 8
SULLIVAN TAX MAP 5 LOTS 46 & 46-1
57 ROUTE 9
KEENE & SULLIVAN, NEW HAMPSHIRE
CHESHIRE COUNTY

OJECT:
GORDON SERVICES
KEENE

TITLE:

RECLAMATION PLAN

PROJECT No. DATE: 23-0201-1 MAY 9, 2025 HORIZ. SHEET: 19 55 23 12=60

STORMWATER MANAGEMENT REPORT





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GORDON SERVICES - KEENE

Keene: Map 215; Lots 7 & 8 Sullivan: Map 5; Lots 46 & 46-1

57 Route 9

Keene & Sullivan, New Hampshire

January 22, 2025 Revised: August 6, 2025 Revised: August 22, 2025

PREPARED FOR:

G2 HOLDINGS, LLC 250 NORTH STREET JAFFREY, NH 03452

PREPARED BY:

GRANITE ENGINEERING, LLC 150 DOW STREET, TOWER 2, SUITE 421 MANCHESTER, NH 03101 603.518.8030

GE Project No. 23-0201-1

I. INTRODUCTION

A. Project Description

The subject properties propose the expansion of an existing gravel and earth removal operation for G2 Holdings, LLC. The properties are located at 57 Route 9 in Keene and Sullivan, New Hampshire. The majority of the site is located within the Keene R (Rural) Zoning District. A proposed gravel road will be constructed to access various points on the site. Stormwater runoff will be managed through a series of sediment basins that connect to an existing infiltration pond.

B. Existing Site Conditions

Keene Tax Map 215 Lot 7 is approximately 78.4 acres in area. Keene Tax Map 215 Lot 8 is approximately 23.1 acres in area. Sullivan Tax Map 5 Lot 46 is approximately 169.0 acres in area. Tax map 5 Lot 46-1 is approximately 28.1 acres in area. The total area of all four subject properties is therefore 298.6 acres in area. The property is currently developed with a gravel removal operation. There are wetlands on the properties to the north and east. There is an existing, previously permitted, stormwater basin located to the south of the property, closest to Route 9.

According to the Site Specific Soil Survey, the predominant onsite soil types are Sunapee, Tunbridge Lyman Rock Outcrop, and Lyman.

Please refer to sections three (3) and eight (8) of this stormwater report for project specific NRCS soils and SSSS report information.

II. STORM DRAINAGE ANALYSIS & DESIGN

A. Methodology

The purpose of this analysis was to determine if the proposed sediment ponds could capture, detain, and release the stormwater flows through small, controlled, outlet pipes to both the existing infiltration area located currently on-site, as well as the proposed infiltration area to be completed during the final phase of the project (Period 8).

In accordance with generally accepted engineering practice, the 2-year, 10-year, 25-year, 50-year and 100-year frequency storm has been used in the various aspects of analysis and design of stormwater management considerations for the subject site. Stormwater-treatment provisions and all drainage facilities have been designed to be fully functional during a 50-year return frequency storm.

In appreciation of the benefits and limitations related to each of the various methods available to design professionals for estimating peak stormwater discharge rates for use in analysis and design, the TR-20 computer model was used. Values for Time of Concentration used in the analysis were estimated using the methodology contained within USDA-S.C.S. publication Urban Hydrology for Small Watersheds Technical Release No. 55 (TR 55).

All proposed stormwater inlet structures were designed to remain under inlet control throughout a design storm of the return frequency noted. Outlet protection for each discharging culvert was designed in accordance with the methodology for the "best management practice", in accordance with a publication entitled New Hampshire Stormwater Manual Volume 2: Post-Construction Best Management Practices Selection and Design. In addition, this publication served as the primary reference for the numerous temporary and permanent erosion control methods incorporated into the design of this project.

All design and analysis calculations performed using the referenced methodologies are attached to this report. The minimum time of concentrations used for the analysis is 6 minutes. These calculations document each catchment area, a breakdown of surface type, time of concentration, rainfall intensity, peak discharge volume, Manning's "n" value, peak velocity, and other descriptive design data for each watershed and pipe segment evaluated. In addition, the "Post Development Drainage Area Plans" graphically define and illustrate the extent of each watershed or catchment area investigated.

B. Post-Development Drainage Conditions

In order to evaluate the impact of the proposed development, two (2) Point of Analysis (POA) was analyzed to demonstrate that the peak rates of runoff would not increase from the site improvements.

The first POA, Link A, is located in the wetlands adjacent to Route 9 and directly south of the proposed project area. Within the wetlands, there is an 18" culvert directing runoff to the southern side of Route 9. This culvert has been shown on DOT Reference plans.

The second POA, Link B, is located in the wetlands directly to the east of the project area. Within the wetlands, there is an box culvert directing runoff to the southern side of Route 9.

Pre-development peak rates of discharge are identified in Table 2. Further explanation of the post-condition hydrology will show a net decrease to the point of analysis.

For a more visual description of the information presented in this section, please refer to the attached "Pre-Development Drainage Areas Plan" attached in the appendix of this report.

The analysis for the development of the site is broken into two segments, Interim and Final. "Interim Development" is in reference to the development of the site from Period 1 through Period 7. Once Period 7 is completed, the project will proceed with Period 8. In this Period, there is an additional excavation in the area of Period 1. For the construction of Period 8, this is viewed as the "Final Development".

Stormwater from within the project area is managed by multiple sediment basins/detention ponds around each work area. These detention ponds are represented in the HydroCAD model and are denoted as SF 5, SF6, and SF7. The intent of the grading of the pit areas, as well as the haul roads, was to keep the stormwater self-contained, with no runoff during a 50-year, 24-hour storm event.

Within the HydroCAD Model, all significant grading for the excavation pits and detention basins was assigned as a grass surface and a hydrologic soil group 'D'.

The detention basins mentioned above are designed to withhold and slowly discharge stormwater runoff to the infiltration basins near the lower portion of the project. During the project, in Period 1, the Infiltration Basin SF1 will be constructed to handle the runoff from the project site and infiltrate into the soil. Once Period 7 is completed, the project will move forward with Period 8. In this Period, Infiltration Basin SF8 will be constructed and will observe the runoff that originally was directed to SF1.

The proposed infiltration area was designed to use exfiltration though the native soils as its only means of outlet. Infiltration rates for the infiltration ponds were calculated by the default method as set forth in Env-Wq 1054.14. The practice is located in an area identified in the Soil Series Survey as Berkshire, Fine Sandy Loam Soils. Using Ksat values for New Hampshire Soils, Soil Scientists of Northern New England, Special Publications No. 5, September 2009, the lowest value associated with Berkshire soils is 0.6 inches per hour. Using a safety factor of 2, the infiltration rate utilized in the drainage analysis is 0.3 inches per hour.

Bedrock monitoring wells SLR3, SLR10, and BRW-12 were used to determine the floor elevation of both the temporary pond in period 1 and the final infiltration pond in period 8, maintaining 6 feet above the water table.

The results of the drainage analysis determined that the stormwater was infiltrated in its entirety during a 50-year, 24-hour storm event. The self-

contained 50-year storm event for both the Interim and Final Development of the project. This was done through capturing stormwater in large sediment basins with small, controlled outlet devices to release stormwater in a controlled manner and by directing stormwater to the infiltration area.

During the 100-yr, 24-hour storm event, both the Interim and Final Development of the project provide a decrease in peak flow rate that discharge to the two points of analysis.

For a more visual description of the information presented in this section, please refer to the attached "Post-Development Drainage Areas Plan" attached in the appendix of this report.

All of these ponds provide adequate storage to offset the peak rates of runoff for the design storms. The detailed hydrologic and hydraulic relationship of each sub-catchment is described within the HydroCAD stormwater modeling, also contained in the appendix of this report.

The peak stormwater runoff rate for the specific storm frequency is presented and analyzed in the subsequent summary section of this report, for the point of analysis (Table 1).

C. Summary:

TABLE 1: CHANNEL PROTECTION REQUIREMENTS

Site Pre-Development vs. Post-Development (Storm Volume in Acre-Feet)								
Analysis	2-Year							
Point	Pre	Interim	Post					
Α	1.011	0.795	0.795					
В	5.037	3.902	3.902					

TABLE 2: PEAK RUNOFF (ENV-WQ 1507.06)

Site Pre-Development vs. Post-Development (Peak Discharge Rate in cfs)										
Analysis Point		2-Year			10-Year		25-Year			
	Pre	Interim	Post	Pre	Interim	Post	Pre	Interim	Post	
Α	4.07	3.47	3.47	11.06	8.71	8.71	17.43	13.39	13.39	
В	19.72	15.86	15.86	61.33	46.94	46.94	101.14	76.24	76.24	

Site Pre-Development vs. Post-Development (Peak Discharge Rate in cfs)									
Analysis Point		50-Year		100-Year					
	Pre	Interim	Post	Pre	Interim	Post			
Α	23.78	17.98	17.98	31.70	23.63	23.63			
В	141.45	105.66	105.66	192.17	142.52	142.52			

TABLE 3: PEAK STORMWATER POND ELEVATION

Site Post Development (Peak Pond Elevation)								
Description	50-Yea	ar	100-Year					
	Post - Interim	Final	Post - Interim	Final				
Stormwater Basin Berm Elevation	874.00	856.00	874.00	856.00				
Peak Water Elevation	873.04	854.40	873.66	855.32				

III. EROSION & SEDIMENTATION CONTROL PROVISIONS

A. <u>Temporary Erosion Control Measures</u>

Temporary erosion and sediment control measures are indicated on the design plans, construction details, general notes and within the drainage report. Although not integral with this stormwater report, due to the size of the proposed development both temporary and permanent erosion control measures will also be specified within the project's Stormwater Pollution Prevention Plan (SWPPP). All erosion control measures specified are designed to reduce or eliminate potential soil migration and water quality degradation, both during and after the construction period.

The following temporary erosion control measures will be implemented;

- Silt Fence and/or Silt Logs
- Erosion Control Blankets on slopes 3:1 and steeper
- Riprap Aprons & Spillway Stabilization
- Turf Establishment Hydroseeding with mulch and tackifiers
- Stone Check Dams
- Temporary Sediment Basins

These temporary erosion control measures are also discussed in the projects. Operation and Maintenance plan contained in the appendices of this report.

In addition to the above-listed erosion control measures, references are made throughout the project documents to the <u>New Hampshire Stormwater Manual; Volume 3: Erosion and Sediment Temporary Controls During Construction</u> for additional measures, as necessary.

B. Construction Sequence

A site-specific construction sequence sensitive to limiting soil loss due to erosion and associated water quality degradation was prepared specifically for this project and is shown on the project plans. As pointed out in the erosion control notes, it is important for the contractor to recognize that proper judgment in the implementation of work will be essential if erosion is to be limited and protection of completed work is to be realized. Moreover, any specific changes in sequence and/or field conditions affecting the ability of specific erosion control measures to adequately serve their intended purpose should be reported to this office by the contractor. Furthermore, the contractor is encouraged to supplement specified erosion control measures during the construction period where and when in his/ her best judgment, additional protection is warranted.

C. <u>Permanent Erosion Control Measures</u>

Similar to temporary erosion control measures, all permanent erosion control measures are indicated on the design plans, construction details, general notes, drainage report, SWPPP and O & M project documents.

The following permanent erosion control measures will be implemented;

- Stone-lined ditches
- Inlet & Outlet Protection Riprap Stabilization
- Stormwater Basins with multi-stage outlets
- Turf Establishment Hydroseeding with mulch and tackifiers

Each of the above-mentioned permanent erosion control measures are designed in a project-specific manner within both state and local regulatory compliance standards.



GORDON SERVICES – KEENE PIT ACID MINE DRAINAGE MONITORING PROGRAM



57 Route 9, Keene, New Hampshire City of Keene Tax Map 215 Block 7 Town of Sullivan Tax Map 583 Lot 46 & 46-1

Prepared For:

Gordon Services 250 North Street Jaffrey, New Hampshire 03452

Prepared By:

FRONTIER GEOSERVICES
127 OLD WARNER ROAD
BRADFORD, NEW HAMPSHIRE 03221

Joel Banaszak, P.G. (603) 748-3715

Jbanaszak@frontiergeoservices.com

August 21, 2025 Frontier Project No. 2024012



1.0 INTRODUCTION

A comprehensive groundwater and surface water monitoring program will be implemented at the Site to evaluate potential impacts from acid mine drainage (AMD). Monitoring will include quarterly sampling at five groundwater wells (BRW-7, BRW-8, BRW-9, BRW-12, and proposed well BRW-13) and at surface water infiltration features, including a downgradient discharge station and an upgradient background station. pH will also be measured monthly at all monitoring wells and drainage basins to provide early detection of AMD.

Baseline conditions will be established through two pre-excavation sampling rounds, and quarterly monitoring will continue in January, April, July, and October. If four consecutive quarters of results confirm that AMD is not being generated, the monitoring frequency may be reduced to biannual sampling in April and October.

All samples will be analyzed for key field parameters (pH, conductivity, oxidation-reduction potential, dissolved oxygen, and turbidity) and dissolved/total metals (arsenic, copper, iron, manganese, nickel, and lead). Results will be compared to NHDES Ambient Groundwater Quality Standards (AGQS) and Env-Wq 1700 Surface Water Quality Regulations. The determination of AMD will be based on trends of decreasing pH and increasing dissolved metals, not isolated exceedances. Monitoring results will be submitted to the City of Keene Community Development Department within forty-five (45) days of collection.

2.0 ACID MINE DRAINAGE DEFINITON

Acid mine drainage is defined by highly acidic water (pH < 6 standard units) that is enriched in dissolved heavy metals, including arsenic, copper, iron, manganese, nickel, and lead.

The identification of AMD under this program will be based on:

- Consistent relationships between low pH values and elevated concentrations of dissolved heavy metals, rather than individual exceedances of standards in isolation.
- Observed trends over time indicating a decrease in pH in conjunction with an increase in dissolved metals concentrations, which together are diagnostic of AMD formation.

3.0 GROUNDWATER MONITORING

Groundwater samples will be collected on a quarterly basis in January, April, July, and October from the following monitoring wells:

- BRW-7
- BRW-8
- BRW-9
- BRW-12
- BRW-13 (proposed; to be installed downgradient of the entire site between the mining operation and Route 9)

Following four (4) consecutive quarters of analytical results demonstrating that AMD is not being produced, the monitoring frequency may be reduced to a biannual schedule in April and October. Pleaser refer to **Figure 1** for a **Monitoring Well Location Map**.

1

4.0 SURFACE WATER AND INFILTRATION FEATURES

All surface water generated from excavation activities will be directed into infiltration basins. Monitoring of these features will include:

- The infiltration feature currently located in the western portion of Period 8.
- Newly constructed infiltration features, which will be incorporated into the program on an iterative basis as the project progresses.
- A downgradient surface water station located at the primary discharge point of the infiltration system.
- An upgradient background surface water station, SWS-6, to provide baseline comparison data.

5.0 SAMPLING PARAMETERS

At each quarterly monitoring location (groundwater wells and surface water stations), the following parameters will be measured:

- Field parameters: pH, specific conductance, oxidation-reduction potential, dissolved oxygen, and turbidity.
- Laboratory analyses: dissolved and total metals, including arsenic, copper, iron, manganese, nickel, and lead via EPA Method 200.8 ICP-MS.

6.0 BASELINE MONITORING

Prior to initiation of excavation activities, two (2) rounds of baseline samples will be collected from all identified monitoring locations. Sampling events will be conducted a minimum of fourteen (14) calendar days apart.

7.0 MONTHLY PH MONITORING

In addition to the quarterly monitoring described above, pH will be measured on a monthly basis at:

- All surface water drainage basins, including newly constructed infiltration features.
- All groundwater monitoring wells (BRW-7, BRW-8, BRW-9, BRW-12, and BRW-13).

This supplemental monitoring provides an early-warning mechanism to identify AMD generation between comprehensive quarterly sampling events.

8.0 APPLICABLE REGULATORY STANDARDS

Applicable standards for the monitoring program are established under two regulatory frameworks:

- Groundwater: NHDES Env-Or 600 Contaminated Site Management, Table 600-1 Ambient Groundwater Quality Standards (AGQS).
- Surface Water: NHDES Env-Wq 1700 Surface Water Quality Regulations.

For groundwater, standards for heavy metals are applied to dissolved concentrations, which are considered the most representative measure of bioavailability. Some heavy metals may naturally occur at concentrations exceeding applicable standards. In such cases, baseline sampling will be used to establish background concentrations. The determination that AMD is contributing to elevated

metals will be based on the observance of increasing dissolved metal concentrations coupled with decreasing pH values, rather than on single exceedances.

Groundwater Quality Standards (Env-Or 600, Table 600-1 AGQS)

Specific Conductance: No Standard

Oxidation Reduction Potential (ORP): No Standard
Dissolved Oxygen: No Numeric Standard (field indicator)

Turbidity: 10 NTU above natural background pH: No Numeric Standard (field indicator)

Arsenic: 5 µg/L Copper: 1,300 µg/L Iron: 300 µg/L

Manganese: 300 μg/L Nickel: 100 μg/L Lead: 15 μg/L

Surface Water Quality Standards (Env-Wq 1700)

Specific Conductance: No Standard

Oxidation Reduction Potential (ORP): No Standard

Dissolved Oxygen: Narrative Criterion (generally ≥ 75% saturation or 5 mg/L to support aquatic life)

Turbidity: 10 NTU above natural background

pH: Narrative Criterion (generally 6.5–8.0 SU; not below 6.0 SU)

Arsenic: 150 μ g/L (a,b) Copper: 2.3 μ g/L (b,c) Iron: 1,000 μ g/L

Manganese: No Numeric Standard

Nickel: 13.3 μg/L (a,c) Lead: 0.41 μg/L (a,c)

Footnotes (from Env-Wq 1700):

- (a) Metals criteria expressed as a function of the Water Effect Ratio (WER). Values shown correspond to WER = 1.0; site-specific criteria shall be determined per EPA (1994) guidance.
- (b) Values for aquatic life protection are expressed as dissolved metals and, for hardness-dependent metals, are based on hardness of 20 mg/L. Conversion between dissolved and total recoverable metals must use the equations and tables in Env-Wq 1703.23.
- (c) Hardness-dependent metals criteria vary with water hardness (mg/L as CaCO3). The values shown correspond to hardness of 20 mg/L. Criteria for other hardness values shall be calculated using Env-Wq 1703.23 and Env-Wq 1703.24.

9.0 DATA REVIEW AND REPORTING

All analytical results, including quarterly sampling data and monthly pH monitoring results, will be reviewed against the applicable groundwater and surface water quality standards described in Section 8.0.

The determination of AMD will be based on observed trends of decreasing pH and increasing concentrations of dissolved heavy metals, in accordance with the definition provided in Section 2.0.

Results will be submitted to the City of Keene Community Development Department within forty-five (45) days of sample collection.



GORDON SERVICES – KEENE PIT BRW-12 SLUG TEST DATA & RESULTS



57 Route 9, Keene, New Hampshire City of Keene Tax Map 215 Block 7 Town of Sullivan Tax Map 583 Lot 46 & 46-1

Prepared For:

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Prepared By:

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August 21, 2025 Frontier Project No. 2024012



1.0 INTRODUCTION

A slug test was performed in monitoring well BRW-12 (2-inch diameter PVC in a 3-inch borehole). The well is 90 ft deep, screened from 30–90 ft bgs. The producing interval extends from 30–78 ft bgs. At the time of testing, static water level was measured at 59.4 ft bgs, yielding an effective saturated thickness of \sim 18.6 ft. The well screen below the water table therefore fully penetrated the saturated thickness during the test.

2.0 METHODS

Head displacement, H(t), was defined as the instantaneous change in water level relative to the static condition. The maximum displacement, H_0 , was identified at the start of recovery, and subsequent measurements ($t \ge t_0$) were normalized as the ratio H/H₀. Recovery data were then plotted as $\ln(H/H_0)$ versus elapsed time (trel), and a straight-line fit was applied over the intermediate range of normalized displacement ($0.2 \le H/H_0 \le 0.8$). This mid-range window is standard practice, as it minimizes the influence of early-time turbulence and late-time storage or boundary effects, producing the most reliable estimate of the aquifer response.

Bouwer & Rice Equation

For a fully penetrating well in an unconfined aquifer, Bouwer & Rice (1976, 1989) give:

$$K = (\alpha \cdot r_c^2 \cdot \ln(R_e/r_w)) / (2L)$$

where:

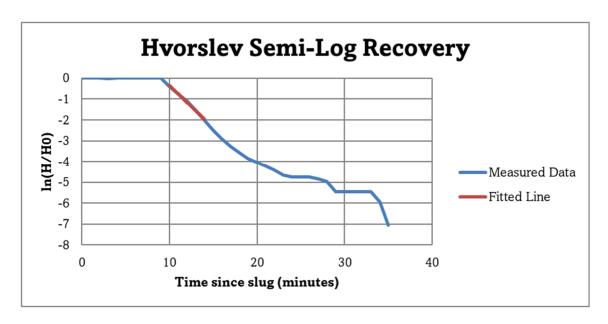
- α = slope of semilog fit (1/min)
- r_c = casing radius (ft)
- r_w = well radius (ft)
- R_e = effective radius of influence (ft)
- L = submerged screen length (ft)

For this test:

- r c = 0.0833 ft
- $r_w = 0.125 \text{ ft}$;
- $R_e = 1.5 \times b_eff = 27.9 \text{ ft.}$
- L = b eff = 18.6 ft;

The regression yielded a decay constant $\alpha \approx 0.4046 \text{ min}^{-1}$ with $R^2 = 0.995$. Substituting into the Bouwer & Rice equation with $\ln(R_e/r_w) = 5.814$, the calculated hydraulic conductivity is:

$$K \approx 0.633 \text{ ft/day } (\approx 0.316 \text{ in/hr})$$



3.0 CONCLUSION

The slug test at BRW-12 indicates an upper-bound hydraulic conductivity of \sim 0.63 ft/day, equivalent to an infiltration capacity of \sim 0.32 in/hr.

4.0 REFERENCES

Bouwer, H., and R.C. Rice (1976). A slug test for determining hydraulic conductivity of unconfined aquifers with completely or partially penetrating wells. Water Resources Research, 12(3), 423–428.

Bouwer, H. (1989). The Bouwer and Rice slug test—an update. Ground Water, 27(3), 304–309.

Butler, J.J., Jr. (1998). The Design, Performance, and Analysis of Slug Tests. Lewis Publishers.

Hvorslev, M.J. (1951). Time Lag and Soil Permeability in Ground-Water Observations. U.S. Army Corps of Engineers, Waterways Experiment Station Bulletin 36.

Outlook

Re: Property Value Analysis Request

From Jim Manley jmanley8@outlook.com>

Date Thu 7/31/2025 9:25 AM

To Mari Brunner < mbrunner@keenenh.gov>

Cc Friedrich K. Moeckel <fmoeckel@tarbellbrodich.com>; aham@keenesentinel.com <aham@keenesentinel.com>; Liz Newcombe <emnewcombe11@gmail.com>; Evan Clements <eclements@keenenh.gov>; Megan Fortson <mfortson@keenenh.gov>; Emily Duseau <eduseau@keenenh.gov>

Ideally this could be done for the 25 August meeting. I cannot see how an objective decision can be reached absent this analysis.

Also reviewing the minutes from the April Conservation Commission meeting a 3D visualization should be rendered, again for full transparency and objectivity.

Jim Manley

Sent from my iPhone

On Jul 31, 2025, at 4:12 PM, Mari Brunner < mbrunner@keenenh.gov > wrote:

Good morning Jim,

We will pass this request on to the Planning Board for their consideration.

Thank you, Mari

----Original Message-----

From: Jim Manley < jmanley8@outlook.com>

Sent: Thursday, July 31, 2025 3:17 AM

To: Mari Brunner < mbrunner@keenenh.gov>

Cc: Friedrich K. Moeckel <fmoeckel@tarbellbrodich.com>; aham@keenesentinel.com; Liz

Newcombe <emnewcombe11@gmail.com> Subject: Property Value Analysis Request

Good morning,

Given a Planning Board member's comment on 28 July that "expert analysis outweighs citizen input "in the approval decision process, I hear-by request a formal analysis of impact to resident property values, particularly those within the 2500 foot "impact zone for well testing" be performed for years 0-14+.

This is a key analysis assessment which appears to be missing to make a fully informed decision.

Thank you for your consideration.

Best regards,

Jim Manley 67 Tyler Lane Sullivan, NH 03445

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